

WSA Northern Gateway Sydney Science Park LUD3 Interim Intersection

Design Report





Prepared for Celestino Developments SSP Pty Ltd 02 April 2024



Document Information

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1 Introduction

Enspire Solutions (**Enspire**) has been engaged by Celestino Developments SSP Pty Ltd (**Celestino**) to prepare the Civil Engineering design and documentation in support of a Development Application (**DA**) submission to Penrith City Council (**PCC**) for a new interim signalised intersection along existing Luddenham Road that will facilitate primary access to the Sydney Science Park precinct (**SSP**).

The site encompasses a section of the existing road reserve on Luddenham Road (approximately 650m) and land within properties on either side of this section as noted below:

- Lot 204 DP 1280188 (Celestino) known as 581 Luddenham Road, Luddenham
- Lot 206 DP 1280188 (Celestino) known as 599 Luddenham Road, Luddenham
- Lot 205 DP 1280188 (Metro)
- Lot 24 DP1277418 (Metro)
- Lot 26 DP1277418 (Metro)
- Road reserve (Penrith City Council)

The proposal is generally referred as 'LUD3 Intersection'.

A general arrangement plan of the Subject Site is shown in Figure 1.

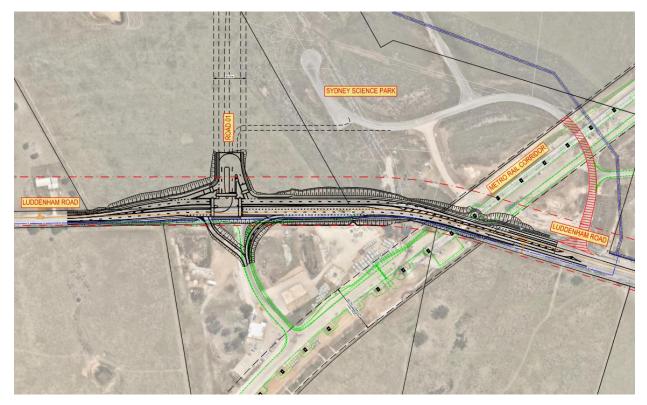


Figure 1 – Subject Site



The proposed development seeks development consent for the following works:

- Removal of trees and vegetation
- Construction of 650m road including the following:
 - Road widening to facilitate dual approach and departure lanes on Luddenham Road within an approximate road reserve width of 20m to 31m including kerbs, medians, traffic islands and footpaths.
 - Provision of a three-way signalised intersection to provide principal access to Sydney Science Park (SSP).
 - Provision of a signalised pedestrian crossing on all approaches of the intersection.
 - Installation of safety barrier, signage, line marking and lighting.
- Construction of access road including slip lanes on the western side of Luddenham Road to provide access to SSP. Construction of internal access track to facilitate access to Sydney Water Corporation Integrated Water Recycling Facility located within SSP.
- Reconstruction of slip lane on eastern side of Luddenham road to maintain construction access to the Metro Viaduct. Reconstruction of temporary left in/left out construction access for Sydney Metro. Construction of a temporary retaining wall to support the reconstructed slip lane.
- Removal and relocation of the overhead and underground electrical services located in the existing road reserve. Note: The intent is to not relocate an existing 132kV line within the existing Luddenham Road reserve; however, this is subject to detailed design.
- Removal and relocation of the underground telecommunication services located in the existing road reserve.
- Reconfiguration of the existing stormwater inlet and outlet headwalls in the existing road reserve.
- Associated demolition works, earthworks, environmental management, civil and stormwater management, and landscaping works.

The DA also seeks consent for construction staging works, as noted below:

- <u>Phase 1:</u> Construct northbound carriageway including access road to Sydney Science Park and carry out west verge electrical relocation.
- <u>Phase 2:</u> Divert traffic to northbound lanes with east lane to operate as a southbound lane temporarily during construction work. Demolish existing Luddenham Road pavement and construct southbound carriageway including Metro construction access road. Carry out telecommunications relocation.

Specifically, the following works are not proposed as part of this application:

- 1. Subdivision of development lots.
- 2. Construction of buildings.



2 Related Reports and Documents

This report is to be read in conjunction with the following reports and documents:

- 1. Development Application civil design drawings prepared by Enspire (Appendix A).
- 2. TfNSW Signals In Principal Approval letter (Appendix B).
- 3. Preliminary TCS Plan, prepared by Road Delay Solutions, dated 20/03/2023 (Appendix D)
- 4. Sydney Science Park Luddenham Road Intersection Transport Assessment, prepared by JMT Consulting, dated 5 April 2023.
- 5. Western Sydney Aerotropolis Precinct Plan May 2023.
- 6. Western Sydney Aerotropolis Development Control Plan 2022.
- 7. Penrith Development Control Plan 2014.
- 8. Austroads Guide to Road Design Part 3 Geometric Design.
- 9. Austroads Guide to Road Design Part 4 Intersections and Crossings General.
- 10. Austroads Guide to Road Design Part 4A: Unsignalised and Signalised Intersections.
- 11. Supplement to Austroads Guide to Road Design Part 3, Publication No: RMS 17.435, prepared by TfNSW (No. TS02642.3).
- 12. Supplement to Austroads Guide to Road Design Part 4, Publication No: RMS 17.335, prepared by TfNSW (No. TS02642.4).
- 13. Supplement to Austroads Guide to Road Design Part 4A, Publication No: RMS 17.336, prepared by TfNSW (No. TS02642.5).



3 The Development

The Subject Site forms part of the Western Sydney Aerotropolis and is located within the Northern Gateway Precinct along existing Luddenham Road. The proposed interim signalised intersection aligns with the key signalised intersection location identified on the Western Sydney Aerotropolis Precinct Plan, Figure 8: Transport Network, dated May 2023 (Refer **Figure 2**).

The interim signalised intersection is a new intersection to be constructed along Luddenham Road and is intended to service the Sydney Science Park development until such time the local government facilitate widening of Luddenham Road to its ultimate 60m wide configuration.

The interim signalised intersection is to replace an existing access road to the north of LUD3 constructed as part of DA16/0176 which has been acquired by Sydney Metro to facilitate construction of the Western Sydney Airport rail link and will be decommissioned as part of the rail works. The interim signalised intersection will ensure continued and uninterrupted access to a Sydney Water Corporation Integrated Water Recycling Hub and provide access to future Sydney Science Park development proposals.

Initial consultation with TfNSW has concluded with the issue of In Principal Agreement (IPA) from TfNSW for the installation of signals on Day 1 of the interim signalised intersection operation and the proposed design does not significantly deviate from the concept design that the IPA was based (refer **Appendix B**).

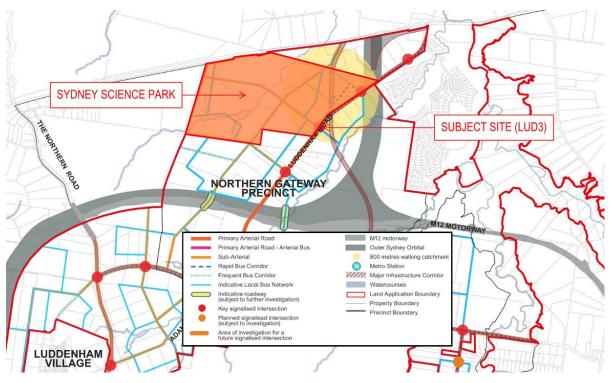


Figure 2 – Subject Site Location

Western Aerotropolis Precinct Plan Figure 8: Transport Network, March 2022



4 Soil and Water Management Plan

This section has been prepared to detail the proposed concept erosion and sediment control strategy and demonstrate general conformance with Part 2.5.5 Erosion and Sediment Control of the Western Sydney Aerotropolis Development Control Plan 2022.

DCP Objectives:

- O1. Protect the health of Wianamatta-South Creek and its tributaries from construction and building runoff and meet the performance criteria for ambient water quality objectives.
- O2. Encourage vegetation retention, protect vegetation during construction and operation, and facilitate prompt rehabilitation through revegetation strategies.
- O3. Minimise site disturbance during construction, reduce the amount of erosion, and stabilise construction works as quickly as possible following completion.

DCP Performance Outcomes:

 PO1. Development is to ensure 80% of all flows leaving the construction site achieves total suspended solids of 50mg/L or less and a pH of 6.5-8.5 during the construction and building phases until the site is stabilised and landscaped.

4.1 **Construction Phasing**

The interim signalised intersection is anticipated to be constructed in two key stages as follows and illustrated in **Figure 3**:

- Phase 1 Construct northbound carriageway including Road 01 connection and carry out west verge electrical relocation. Traffic to remain on existing Luddenham Road carriageway.
- Phase 2 Divert traffic to northbound lanes with east lane to operate as a southbound lane temporarily during construction work. Demolish existing Luddenham Road pavement and construct southbound carriageway including Metro construction access road and temporary retaining wall. Carry out telecommunications relocation. Install and commission traffic signals and install signage and linemarking.

This anticipated construction phasing will minimise traffic disturbance and traffic control measures while also separating the works into smaller areas of land disturbance enabling for more effective erosion and sediment control measures to be implemented as detailed in the following sections.

The proposed construction phasing is conceptual and subject to change as part of detailed design and coordination with the contractors preferred construction methodology.



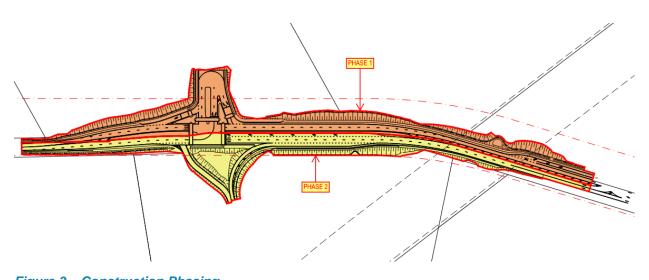


Figure 3 – Construction Phasing Construction phasing extents may change as part of detailed design and to align with the contractors preferred methodology.

4.2 Erosion and Sediment Control Measures

The proposed erosion and sediment control measures are to conform generally with the Landcom Manual, "Managing Urban Stormwater Soil & Construction" 2004 (Blue Book) during all phases of construction and following completion of the works. Control measures are conceptually detailed on drawings 180001-01-DA-C03.01 to 180001-01-DA-C03.22 and include but are not limited to:

- 1. Installation of sediment fencing immediately downstream of disturbed areas including stockpiles to intercept stormwater runoff from the work area prior to entering waterways or stormwater inlets.
- 2. Inspection and maintenance of sediment fencing following rainfall.
- 3. Redirecting external undisturbed catchment runoff away from disturbed areas to minimise erosion potential by reducing runoff through the work area.
- 4. Construction of temporary sediment control basins to collect sediment laden stormwater runoff from the worksite.
- 5. Flocculation of sediment control basins to ensure settlement of dispersive soils within 5 days of rainfall.
- 6. Applying appropriate dust control measures such as spraying disturbed areas to keep damp but not saturated to minimise sediment mobilisation.
- 7. Applying hydroseed progressively to disturbed unpaved surfaces to minimise erosion until final landscaping can be applied.
- 8. Applying turf and vegetation cover immediately following completion of works for long term erosion control.



4.3 Calculations and Estimated Outcomes

A construction phase catchment analysis has been undertaken to determine achievement of the Objectives and Performance Outcomes adopting the proposed erosion and sediment control measures and is detailed below and summarised in **Table 1**.

Catchment ID	Catchment Area (m²)	Estimated Annual Soil Loss ¹ (t/year)	Estimated Soil Loss During Construction ² (m ³)	Proposed Sediment Basin Storage Volume ³ (m ³)	Nominal Total Suspended Solids Concentration⁴ (mg/L)				
P1C1	4,700	47	5	5	<50mg/L				
P1C2	6,300	55	6	6	<50mg/L				
P1C3	3,800	32	4	4	<50mg/L				
P2C2	4,800	29	5	5	<50mg/L				
P2C3	6,500	57	9	9	<50mg/L				
P2C1	2,000	Catchment is less than 2,500m ² and is to conform with The Blue Book controls ⁵							
P2C4	2,300	Catchment is less than 2,500m ² and is to conform with The Blue Book controls ⁵							

Table 1 – Erosion and Sediment Control Design Summary

Explanatory Note 1

Estimated annual soil loss has been calculated adopting the Revised Universal Soil Loss Equation (RUSLE) which is an international industry standard equation for estimating average annual soil loss per unit area. The coefficients to appropriately apply this equation have been adopted from The Blue Book which provides the necessary data for the local catchment. The values calculated represent average soil loss over a 12-month period and is not reflective of total soil loss for a particular construction activity.

Explanatory Note 2

The interim signalised intersection is proposed to be constructed over a 6-month period with an assumed commencement in June 2024. Phase 1 is estimated to be undertaken over 3 months and Phase 2 is estimated to be undertaken in the remaining 3 months. Based on this and adopting monthly rainfall distributions from The Blue Book (refer **Figure 4**), the calculated annual soil loss has been adopted to inform estimated soil loss for the applicable construction duration and converted from tonnes to cubic meter units.

Zone	Ja	in	Fe	eb	м	ar	Α	pr	м	ay	J	un	J	u	A	ug	s	ер	C	ct	N	ov	D	ec
1	6	6	7	8	8	8	6	5	5	4	3	2	2	2	2	2	2	2	2	3	3	4	4	4
2	10	9	9	8	7	5	2	2	1	1	2	1	1	1	1	1	3	3	3	4	5	6	7	8
3	6	8	9	9	10	7	7	4	2	2	2	2	2	1	0	1	2	2	2	3	3	4	6	6
4	6	6	8	8	8	5	5	3	3	2	2	2	2	3	3	2	2	3	3	3	5	5	5	6
5	2	3	7	13	13	10	11	6	3	2	3	2	2/	2	1	1	1	3	3	A	3	2	2	2
6	11	10	10	9	6	5	2	2	2	1	1	1	1	1	1	1	2	2	4	В	5	5	8	7
7	9	9	7	8	4	5	3	3	2	3	2					2	2	3				6	7	7
8	7	8	7	8	5	6	4	3	2	2	2	٦٢		SE	1	2	2	2	PH	AS	ΕZ	6	7	7
9	8	9	8	7	6	5	3	3	2	2	1		14	1%		2	3	3		21%	0	6	6	6
10	7	6	9	7	7	6	4	4	3	2	1	1	2	1	1	2	2	3	4	5	6	6	5	6
11	10	11	11	9	10	5	3	1	1	1	1	1	1	1	2	2	1	2	2	5	6	6	6	6
12	10	9	8	7	5	4	4	2	2	1	1	2	1	1	1	2	3	4	3	4	4	6	7	9

Table 6.2 Percentage of average annual EI that normally occurs in the first and second half of each month for each Rainfall Zone (figure 4.9) (Rosewell and Turner, 1992)

Figure 4 – Adopted Rainfall Distribution

Source: The Blue Book.



Explanatory Note 3

Proposed sediment basin storage volumes are proposed to match the estimated construction soil loss volumes to effectively capture and hold all estimated sediment based on average weather conditions. This design rationale exceeds the Performance Outcome to treat only 80% of flows during construction and provides some contingency for above average weather conditions and small isolated sub catchments that may bypass the sediment control basins. Sediment control basin settlement zone volumes have been calculated based on 5-day 80th percentile rainfall depths from The Blue Book which is appropriate given the estimated duration of works is 6 months or less and the works are not located directly adjacent a highly sensitive waterway.

Explanatory Note 4

As the sediment control basin storage volumes are proposed to match the average soil loss for the construction duration, the concentration of sediment laden water leaving the construction site is estimated to be effectively zero on average for catchments greater than 2,500m². This assessment assumes all suspended solids from captured runoff is to be removed by the contractor prior to pump out which is unlikely, and it is expected that the contractor will pump out captured water only where testing indicates the total suspended solids concentration is less than or equal to 50mg/L and pH levels are between 6.5-8.5.

Explanatory Note 5

Sediment control basins are not proposed for catchments P2C1 and P2C4 due to site constraints and only sediment control fencing and site stabilisation techniques are proposed. Section 6.3.2(d) of the Blue Book advises that sediment basins may not be required for small disturbance areas (<2,500m²) provided the estimated annual soil loss volumes are low (<150m³/year). Annual soil loss volumes for catchments P2C1 (2,000m²) and P2C4 (2,300m²) are anticipated to be less than 12m³/year which is below the 150m³/year threshold which can be adequately managed with erosion and sediment control techniques other than sediment control basins.

4.4 Installation and Maintenance

The estimated performance of the proposed erosion and sediment control measures assumes proper installation and effective and regular maintenance of the controls. Details for each of the proposed control measures in addition to an indicative maintenance schedule is provided on drawing 180001-01-DA-C03.22 to guide the contractor on minimum standards to be implemented during the construction phase.

4.5 Concurrent Transport for NSW Works

As indicated by aerial imagery on drawing 180001-01-DA-C01.41, Transport for NSW have commenced construction of the Sydney Metro – Western Sydney Airport rail line. Due to this, the nature of site conditions on Lot 24 are likely to vary at construction commencement of LUD3 and during construction. Management of construction stormwater runoff is to be coordinated with Transport for NSW at the time of construction and formalised through preparation of a Construction, Environment Management Plan.



5 Earthworks

This section has been prepared to detail the proposed earthworks and retaining wall strategy and demonstrate general conformance with Part 2.18 Earthworks and Retaining Walls of the Western Sydney Aerotropolis Development Control Plan 2022.

DCP Objectives:

None.

DCP Performance Outcomes:

- PO1. To ensure site planning considers the stability of land, its topography, geology and soils.
- PO2. To ensure that earthworks and retaining wall construction is suitably designed and landscaped to ameliorate its visual presentation to and from the public domain and adjacent properties.
- PO3. To encourage reuse of fill material from within the Aerotropolis Precinct.

5.1 Cut and Fill Operations

As part of the proposed works, bulk earthworks on the site will generally consist of cut and fill operations to establish the proposed road formation and batter slopes up to 1V in 4H.

Approximate cut to fill earthworks operations for the works subject to this development application are summarised in **Table 2**.

Earthworks	Volume (m ³)
Cut	-16,150
Fill	+10,450
Balance	-5,700 (export)

Table 2 – Estimated Cut and Fill Volumes

The cut and fill earthworks volumes provided are concept only and are subject to change pending final coordination and detailed design. Cut and fill volumes were estimated based on the following assumptions:

- Allowance made for 150mm topsoil stripping.
- Allowance made for 500mm removal of existing pavement.
- Allowance made for 800mm proposed pavement boxing.
- Allowance made for 100mm topsoil replacement across landscape areas.
- No allowance for earthworks bulking factors.
- No allowance for soil generated from utility service and stormwater drainage trenching.

Excess material suitable for reuse is to be stockpiled on the Sydney Science Park site and covered for future use. Excess material not suitable for reuse is to be disposed of offsite at an appropriate disposal facility.

All unpaved disturbed surfaces are to be landscaped in accordance with the landscape architect's plans.



6 Geometric Design

6.1 **Design Parameters**

The interim signalised intersection has been generally designed in accordance with Austroads Guide to Road Design and TfNSW Supplements to the Austroads Guide to Road Design. A summary of design parameters is provided in **Table 3** and detailed in **Appendix C**.

Design Element	Design Parameter				
Lu	ddenham Road (Major Road)				
Operating Speed	80km/hr				
Design Speed	90km/hr				
Reaction Time	1.5 seconds ¹				
Observation Time	3.0 seconds				
Design Vehicle	20.0m ARV				
Check Vehicle	26.0m B-Double				
Approach Sight Distance (ASD) ²	126m (cars) – to a 0m object height				
Stopping Sight Distance (SSD)	126m (cars) – to a 0.2m object height				
Safe Intersection Sight Distance (SISD)	201m (cars)				
	Road 01 (Minor Road)				
Operating Speed	60km/hr				
Design Speed	70km/hr				
Reaction Time	1.5 seconds				
Observation Time	3.0 seconds				
Approach Sight Distance (ASD)	83m (cars) – to a 0m object height				
Stopping Sight Distance (SSD)	83m (cars) – to a 0.2m object height				

Table 3 – Interim Signalised Intersection Design Parameters

1. Reaction time based on alert driving conditions due to presence of traffic signals. Additionally, TfNSW Supplement to Austroads Guide to Road Design nominates a 1.5 second reaction time for road speeds up to 90km/hr.

2. ASD cannot be achieved for the Major Road hence SSD has been adopted as per Section 3.2.1 of Austroads Guide to Road Design Part 4A.

6.2 Intersection Arrangement

The interim signalised intersection arrangement is depicted in **Figure 5** and incorporates the following elements:

- Dual lane approaches and departures.
- 150m long single right turn lane from Luddenham Road to Road 01.
- 55m long single left turn slip lane from Luddenham Road to Road 01.
- Dual right turn lanes from Road 01 to Luddenham Road.
- Dual left turn lanes from Road 01 to Luddenham Road.
- 100m long left turn deceleration lane on Luddenham Road on the south bound approach for private construction access (Sydney Metro).
- Left turn private construction exit on the south bound departure (Sydney Metro).

In addition, the proposed design has adopted minimum 3.5m wide travel and turn lanes, 1.0m to 2.0m wide shoulders (narrower shoulder adopted at transitions to existing Luddenham Road) and 0.0m to 2.0m wide berms for utilities such as lighting (berm tapers to edge of pavement at transitions to existing Luddenham Road, to match existing road formation).



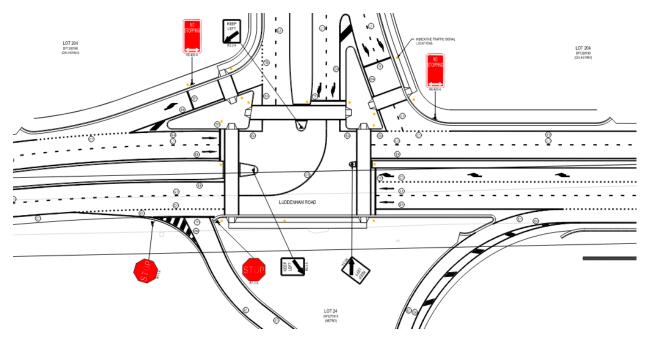


Figure 5 – Intersection Layout

6.3 Superelevation

An assessment of the proposed horizontal road geometry relative to the design speed of Luddenham Road indicates superelevation is necessary and recommended for safety at road bends and at the traffic signals junction respectively. The application of superelevation has been designed assuming a side friction factor of 0.15 (refer **Figure 6**). Adopting a superelevation of 4% on horizontal curves indicates a minimum horizontal curve radius of 336m is to be adopted. The minimum horizontal curve proposed along Luddenham Road (80km/hr Operating Speed) is 350m which is greater than the minimum required to achieve adequate side friction for both cars and trucks.

$R = V^2 / [127^*(e+f)]$
$R = 90^2 / [127^*(0.04 + 0.15)]$
R = 336m

Table 7.5: Recommended side friction factors for cars and trucks												
	f											
Operating speed (km/h)	Ca	ars	Tru	cks								
(,	Des max	Abs max	Des max	Abs max								
40	0.30	0.35	0.21	-								
50	0.30	0.35	0.21	0.25								
60	0.24	0.33	0.17	0.24								
70	0.19	0.31	0.14	0.23								
80	0.16	0.26	0.13	0.20								
90	0.13	0.20	0.12	0.15								
100	0.12	0.16	0.12	0.12								
110	0.12	0.12	0.12	0.12								
120	0.11	0.11	0.11	0.11								
130	0.11	0.11	0.11	-								

Note: ARRB research into the stability of high centre of gravity articulated vehicles indicated that the least stable vehicles may roll over at side friction values as high as 0.35 (Mai & Sweatman 1984).

Figure 6 – Side Friction Values

Source: Austroads Guide to Road Design Part 3: Geometric Design.



Single cross fall is proposed to be continued beyond the horizontal curve between approximate Luddenham Road Chainage 100 to Chainage 350 which improves operation of the intersection during green phases, improves intersection approach sightlines and is more consistent with the natural topography.

6.4 Sight Distances

Sight distance checks have been undertaken in accordance with Austroads Guide to Road Design Part 3: Geometric Design and Part 4A: Unsignalised and Signalised Intersections and have been demonstrated on drawings 180001-01-DA-C07.01 to 180001-01-DA-C07.06. A reaction time of 1.5 seconds has been adopted based on TfNSW Supplement to Austroads Guide to Road Design in calculating stopping distances.

6.5 Vertical Clearances

Vertical clearance to the Sydney Metro viaduct has been based on AS5100 which requires a minimum 5.4m clearance to arterial roads. Vertical clearances achieved are provided on drawings 180001-01-DA-C07.02 and 180001-01-DA-C07.04 and demonstrate that the minimum clearance can be achieved.

6.6 Future Luddenham Road Widening

The proposed interim signalised intersection design has adopted an alignment, typical road cross section and vertical geometry between approximate Luddenham Road Chainage 160 to Chainage 350 that has high potential to be retained as part of the future Luddenham Road widening works and does not preclude the construction of the future road widening.

The proposed alignment of this section of Luddenham Road has been designed to be parallel to the 60m wide road widening corridor with the carriageway pavement offset from the future east boundary consistent with the location of the future kerb and channel lip as per the Western Sydney Aerotropolis DCP typical section for a 60m wide road corridor.

The proposed vertical geometry for this section of Luddenham Road is generally fixed in both the interim and ultimate scenarios due to the sight distance requirements detailed in **Section 6.4** combined with the vertical clearance constraints highlighted in **Section 6.5**. The proposed vertical geometry for the interim signalised intersection (including superelevation and single cross fall for vehicle side friction compliance) between approximate Chainage 160 to Chainage 350 therefore has high potential to be adopted as part of the future Luddenham Road widening to minimise traffic disturbance and overall construction cost.

6.7 Turn Paths

The proposed design vehicle for the interim signalised intersection is the 20.0m Articulated Rigid Vehicle in accordance with AS2890.2 while the check vehicle adopted is the 26.0m B-Double in accordance with AS2890.2.

Turn paths have been generated using the AutoTURN software package and have adopted Austroads Guide to Road Design Part 4: Intersections and Crossings – General Section 5.6 recommendations.

Turn paths are provided on drawings 180001-01-DA-C25.01 to 180001-01-DA-C25.03.



6.8 Viaduct Column Horizontal Clearance

The interim horizontal clearance from the edge of proposed travel lanes to the viaduct columns is a minimum 6.7m. The natural ground levels surrounding the viaduct columns is to be maintained, positioning the base of the columns approximately 1m higher than proposed road levels with a 1 in 4 batter slope. Based on Austroads Guide to Road Design Part 6, adopting previous clear zone concepts, the clear zone in the immediate area is 6.5m. Based on this initial assessment, no vehicle barrier systems are proposed.



7 Stormwater Management

This section has been prepared to detail the proposed stormwater management strategy and demonstrate general conformance with the following subsections of the Western Sydney Aerotropolis Development Control Plan 2022:

- Part 2.3 Stormwater, Water Sensitive Urban Design and Integrated Water Management.
- Part 2.5.1 Flood Management.

Given the proposal only relates to construction of roads and associated works, the nature of the proposed development is categorised as public infrastructure and only limited provisions are applicable. These matters are addressed below in detail.

7.1 DCP Part 2.3 Stormwater Management

7.1.1 Part 2.3.1 Waterway Health and Riparian Corridors

The proposed interim signalised intersection forms part of the Cosgroves Creek catchment and is located at the top of one of its Strahler Order 1 tributaries. It is noted however that there is an inconsistency in the spatial data (Six Maps) and the existing topography from detail survey as depicted in **Figure 7**. There is a natural ridge line adjacent existing Luddenham Road which the spatial mapping has incorrectly identified a stream crossing over. In this respect the detailed survey prevails and the impact that the development (with implementation of the proposed construction and post construction controls) will have on existing waterways will be negligible.

The proposed interim signalised intersection is not located at or beyond a 15ha catchment threshold in a Strahler Order 1 tributary and therefore no riparian corridor restoration works are required.

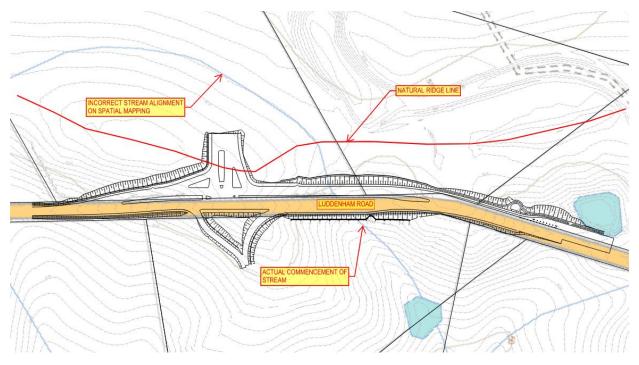


Figure 7 – Strahler Order 1 Stream Alignment



7.1.2 Part 2.3.2 Stormwater Management and WSUD

An assessment of the WSA DCP Part 2.3.2 Benchmark Solutions has identified that the interim signalised intersection development typology (Infrastructure) has not been considered and that the Benchmark Solutions have been calibrated based on assessment of Large Format Industrial (LFI), High Density Residential (HDR), and Low Density Residential (LDR) only in accordance with the Technical guidance for achieving Wianamatta South Creek stormwater management targets (DPE, 2022).

Given the nature of Infrastructure development, application of the current WSA DCP Part 2.3.2 controls is not appropriate and in the absence of guidance in the DCP or supporting technical documents for this development typology, alternative measures are proposed as detailed in the following sections.

7.1.2.1 PO1 Stormwater Quality

Due to spatial constraints in Infrastructure development, utilisation of residual existing grass lined swales and buffers will be relied upon to meet the intent of stormwater quality improvement in the interim. This strategy is proposed for the interim infrastructure scenario only and achievement of the WSA Part 2.3.2 Benchmark Solutions will be achieved as part of the ultimate regional stormwater management strategy or as part of future interim development where adjacent subdivision and/or lot development is proposed.

7.1.2.2 PO2 Stormwater Flow Targets

To assess stormwater flow impacts for Infrastructure development, The Penrith City Council DCP 2014 part C3 has been adopted as a guide and requires an assessment of pre-development and post-development peak stormwater flows to determine if the impact of development will have detrimental effects to the surrounding environment. Peak flows have been assessed using the DRAINS software package at the control points nominated in **Figure 8** and outcomes summarised in **Table 4**.



Figure 8 – Peak Flow Control Points



As indicated in **Table 4**, the effect of localised storages at inlet headwalls and pits in the postdevelopment scenario contributes to generating negligible increases in peak flows at each of the control points. It is noted that the estimated impacts are temporary and do not affect lands currently used for residential or sensitive purposes, with the ultimate post development flows to be appropriately managed through regional stormwater infrastructure operated by Sydney Water Corporation adjacent Cosgroves Creek. Further, it is generally not feasible as part of road infrastructure development to provide attenuation infrastructure due to the narrow working corridor and there is limited flexibility in an interim scenario to alter the pit and pipe network to discharge to temporary detention basins without requiring these assets including road pavements and services to be reconstructed to the ultimate configuration in future.

The DRAINS model indicates that the capacity of pipe crossings (both existing and proposed) is not exceeded and that the proposed design generates reductions in the quantity of stormwater overtopping Luddenham Road at CP1 in the 1% AEP.

Control Point	1% AEP Pre-Development Flow (m³/s)	1% AEP Post-Development Flow (m³/s)	% Difference
CP1	0.93	0.95	+2.5
CP2	0.46	0.39	-14.7
CP3	0.95	1.07	+12.3
Total ¹	2.13	2.34	+9.9

Table 4 – Peak Flow Assessment

1. Total values differ to the sum of CP1 to CP3 due to differences in peak flow timing.

7.1.2.3 PO4 Streetscape Measures

The proposed development does not include regional stormwater infrastructure and does not preclude the achievement of PO1 or PO2 targets in the ultimate Luddenham Road design (i.e. landscaping and stormwater management in accordance with the WSA DCP can be provided as part of future ultimate LUD3 works). Existing grass lined swales and buffers will be relied upon in the interim to address PO1 and PO2.

7.1.2.4 PO7 Safe Overland Flow Paths

The proposed stormwater management system has been modelled in the DRAINS software package and all stormwater flows within the carriageway and roadside swales are contained in the 1% AEP to safe depth and depth x velocity product. During larger storm events or where excessive blockage occurs, overtopping of the roadside swales will occur which mimics the predevelopment condition.

7.2 DCP Part 2.5.1 Flood Management

The proposed interim signalised intersection is not located within or adjacent existing floodway or flood storage area and as demonstrated in Section 7.1.2.2 will have minimal impact on local peak flows and negligible impact overall on Cosgroves Creek flood behaviour. **Figure 9** shows the location of the proposed development relative to existing 1% AEP flood extents.



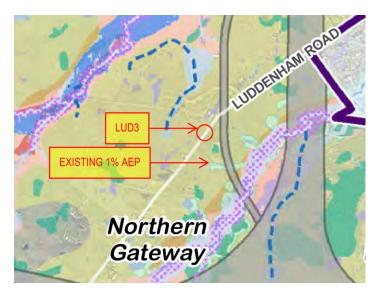


Figure 9 – Existing 1% AEP Flood Extent Underlay

Source: Western Sydney Aerotropolis (Initial Precincts) Stormwater and Water Cycle Management Study, prepared by Sydney Water Corporation, December 2021 Figure 3-1.



8 Pavements

Preliminary pavement profiles indicating anticipated pavement designs have been provided in the Enspire drawing package including a pavement layout to indicate the pavement design intent. Final pavement design details are to be provided by a Geotechnical Engineer as part of detail design taking into consideration the pavement extents nominated.

The concept pavement design consists of:

- 1. F1. Flexible pavement profile generally in accordance with a TfNSW pavement design for arterial roads for sections of the LUD3 intersection that have potential to be adopted as part of the ultimate Luddenham Road widening (approximate Luddenham Road Chainage 160 to Chainage 350 as per **Section 6.6** discussion).
- 2. F2. Flexible pavement profile generally in accordance with Austroads Guide to Pavement Technology and Penrith City Council specifications for transitions between pavement type F1 and existing Luddenham Road (i.e. have no potential to be adopted as part of the ultimate Luddenham Road widening).
- 3. Concrete medians.
- 4. Concrete footpaths.
- 5. Flexible pavement profile generally in accordance with Austroads Guide to Pavement Technology for the temporary Sydney Metro construction access.

All pavements are proposed to be constructed in accordance with Penrith City Council construction specifications.



9 Retaining Walls

Through negotiations with Transport for NSW with respect to impacts on existing Lot 24 and Lot 26, in principle support of the proposed design is predicated on impacts other than the immediate construction entry and exit works being wholly contained within the future Luddenham Road widening corridor. To achieve this outcome, a retaining wall is proposed between approximate Luddenham Road chainage CH270 to CH395 with an indicative elevation provided on drawing 180001-01-DA-C15.01. The wall is likely to be of masonry construction due to the height and surcharge loading anticipated which is to be refined as part of detail design.



10 Utilities

This section has been prepared to detail the proposed concept utilities adjustment strategy and demonstrate general conformance with Part 2.11 Services and Utilities of the Western Sydney Aerotropolis Development Control Plan 2022.

DCP Objectives:

- O1. Ensure the construction of utility services/infrastructure provision occurs in a logical and staged manner, and in sequence with development.
- O2. Encourage innovative and sustainable utility and servicing across the Aerotropolis to promote effective and efficient delivery of services. Ensure utilities designs and locations consider space for alternative future services.
- O3. Design and provide utility infrastructure to integrate with and not negatively impact use of the public realm, liveability, and the environment.
- O4. Infrastructure (new and existing) is protected from the impacts of urban development.

DCP Performance Outcomes:

- PO3. Infrastructure is adequately protected from development.
- PO4. Shared utility trenches combine multiple utilities within a compact area of the street verge, and futureproof service location within road cross-sections.

10.1 Utility Adjustments

The concept design proposes adjustments to existing overhead and underground electrical assets and underground telecommunications. The relocations are proposed to position these utilities outside the new pavement extents and with appropriate vertical cover to suit the new road levels. The relocation works are anticipated to be temporary until the construction of the ultimate Luddenham Road configuration and hence no shared utility trenches are proposed. Not withstanding this, the proposed adjustments complies with the objectives in that provision occur in a logical and staged manner to account for the future widening of Luddenham Road.

New lighting is anticipated to be required for safety and will be installed concurrently where possible with the relocations and positioned beyond the paved road shoulder.

Adjustments to existing 132kV underground electrical and DN450mm potable water main are not proposed however engagement with the relevant service authority may be necessary to facilitate temporary or permanent protection measures as part of the proposed works. These utilities are currently under construction and For Construction details of these utilities have been adopted in the design of the interim signalised intersection.



11 Conclusion

This Civil Engineering and Stormwater Management Report has been prepared to provide an understanding of the design assumptions, inputs and guide to the civil engineering and stormwater management techniques for the proposed interim signalised intersection (LUD3) as depicted in **Figure 1**.

General conformance with the relevant requirements of the Western Sydney Aerotropolis Development Control Plan 2022 has been demonstrated with respect to indicative location, the proposed erosion and sediment controls, earthworks strategy, geometric design, and utility adjustments.

The WSA DCP provides guidance for built form outcomes and does not consider the type of development proposed, being provision of public roads only. Given the proposal does not involve any built form elements, utilisation of residual existing grass lined swales and buffers will be relied upon to meet the intent of stormwater quality improvement on the basis that the WSA DCP Benchmark Solutions have not been calibrated for the subject development typology (Infrastructure) and as such, are deemed not appropriate to implement for this interim development. Notwithstanding, the proposed interim intersection development does not preclude the achievement of the WSA DCP controls as part of the wider future precinct development.

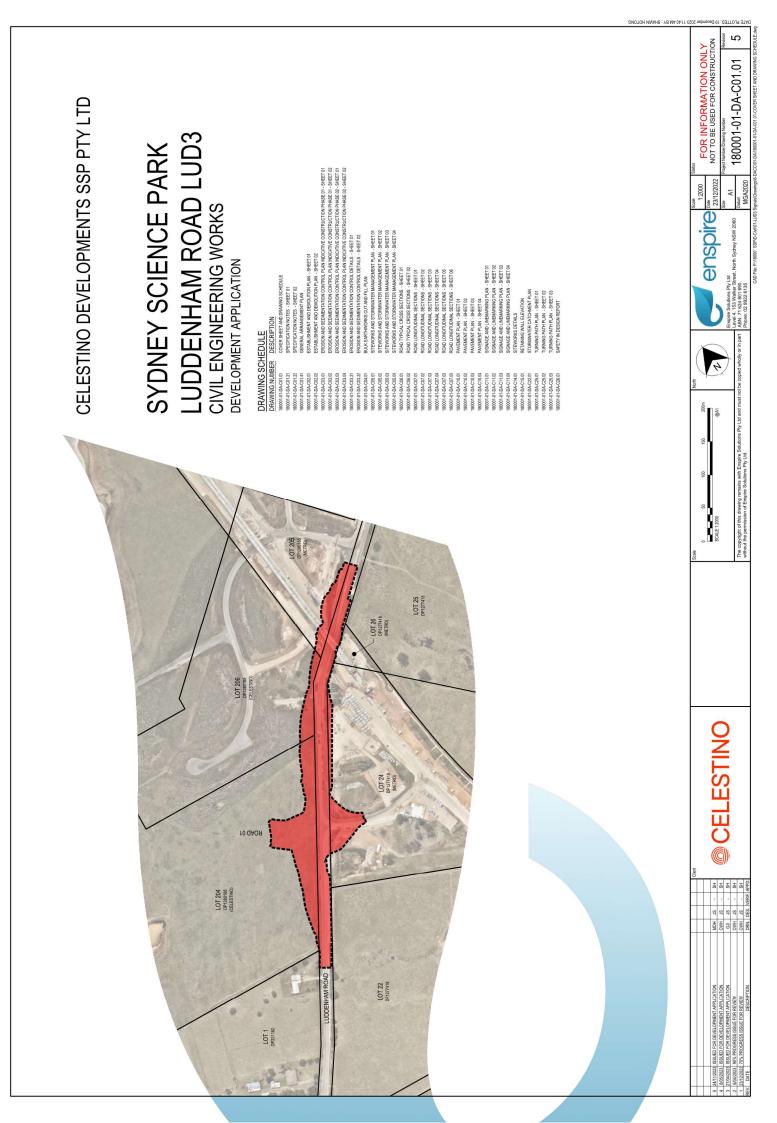
This report demonstrates that the road design requirements of the WSA DCP, PCC DCP, Austroads Guide to Road Design and TfNSW Supplements to Austroads Guide to Road Design can generally be achieved and that post development stormwater runoff can be safely managed. In addition, recommendations from site specific assessment reports such as Bushfire and Flora and Fauna can be accommodated.



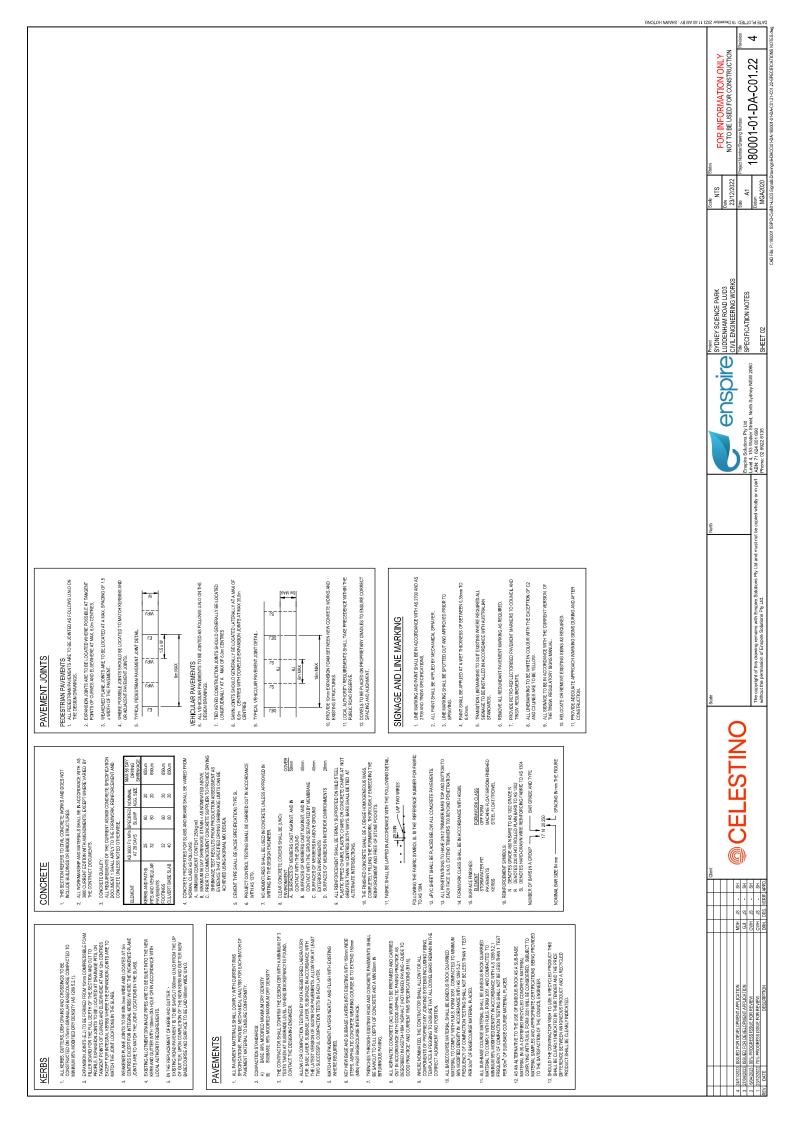
Appendix A Civil Design Drawings

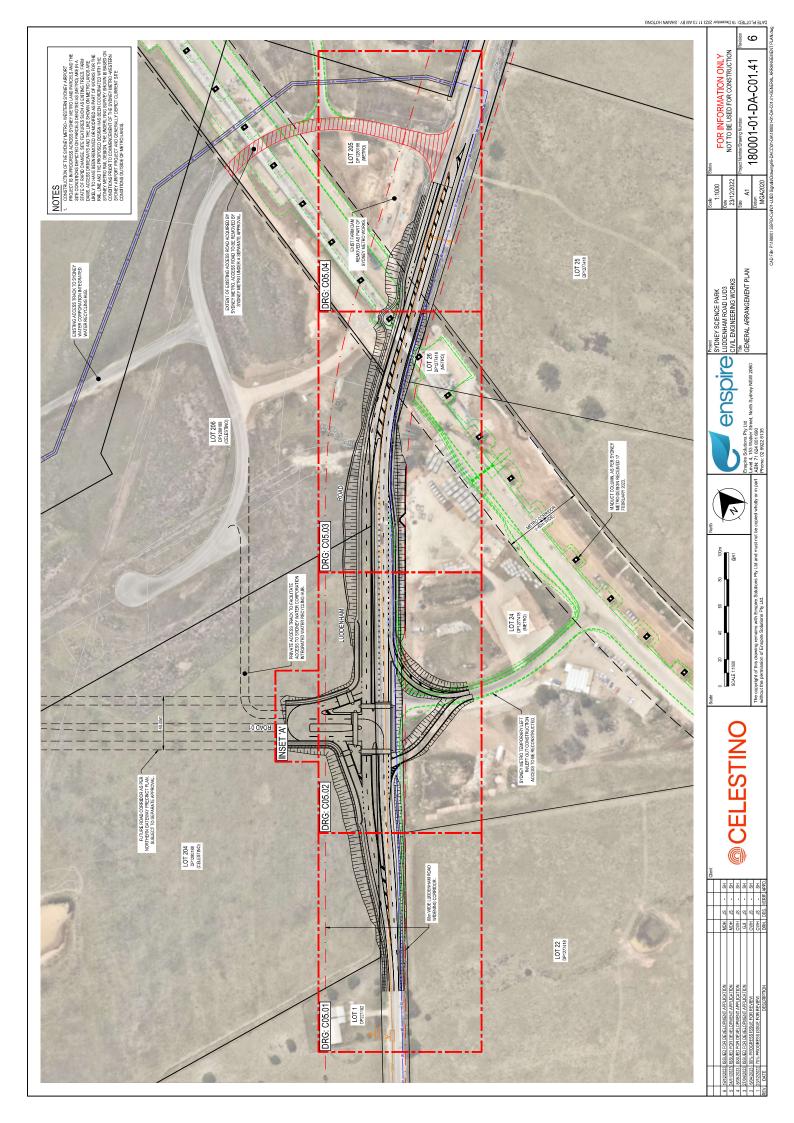
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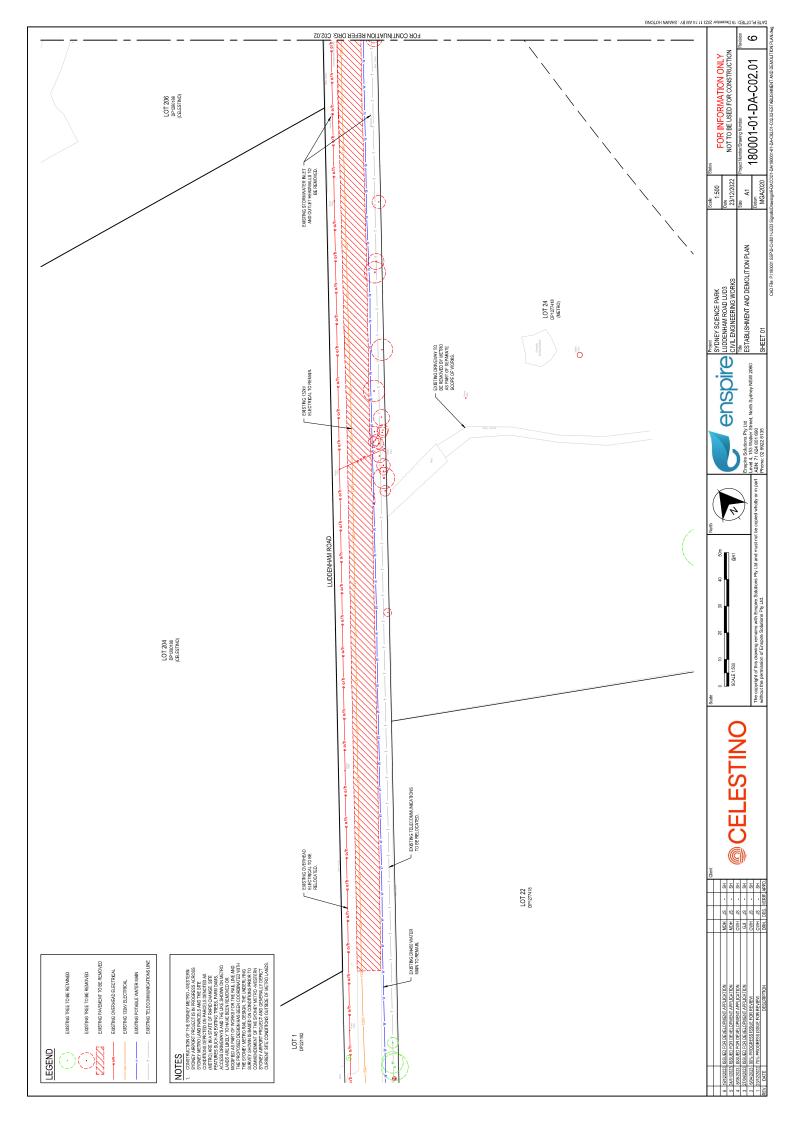


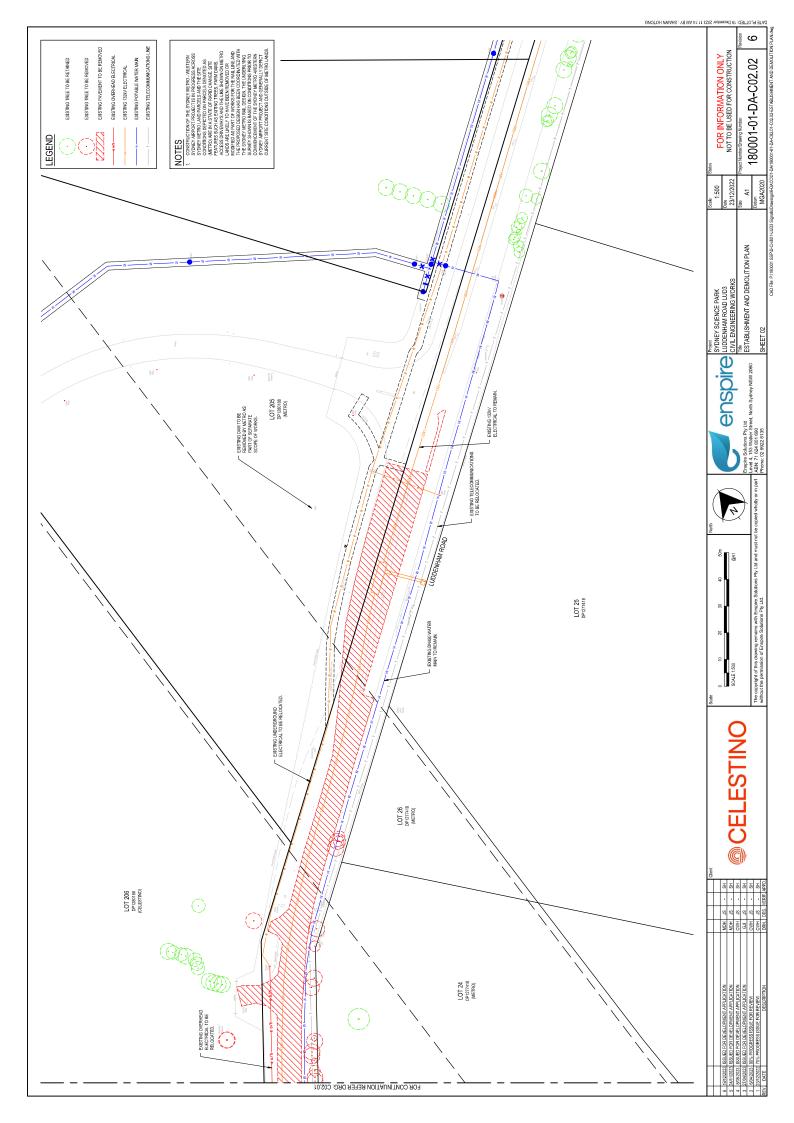


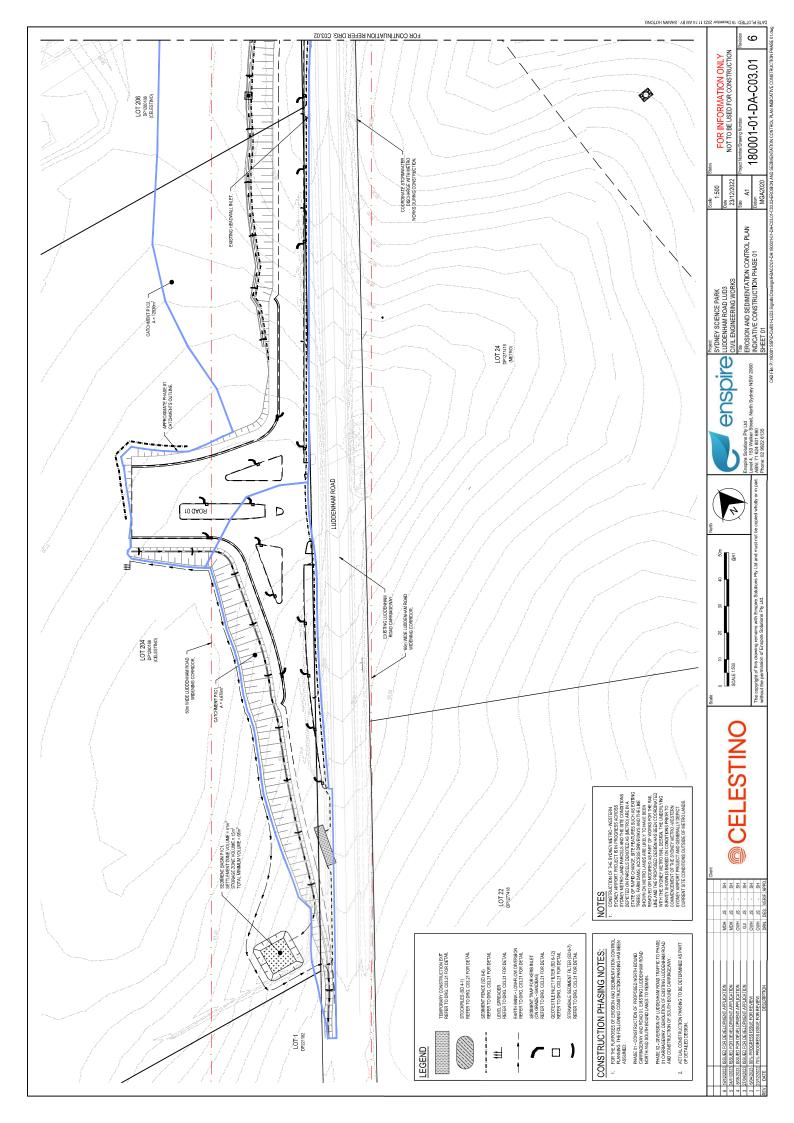
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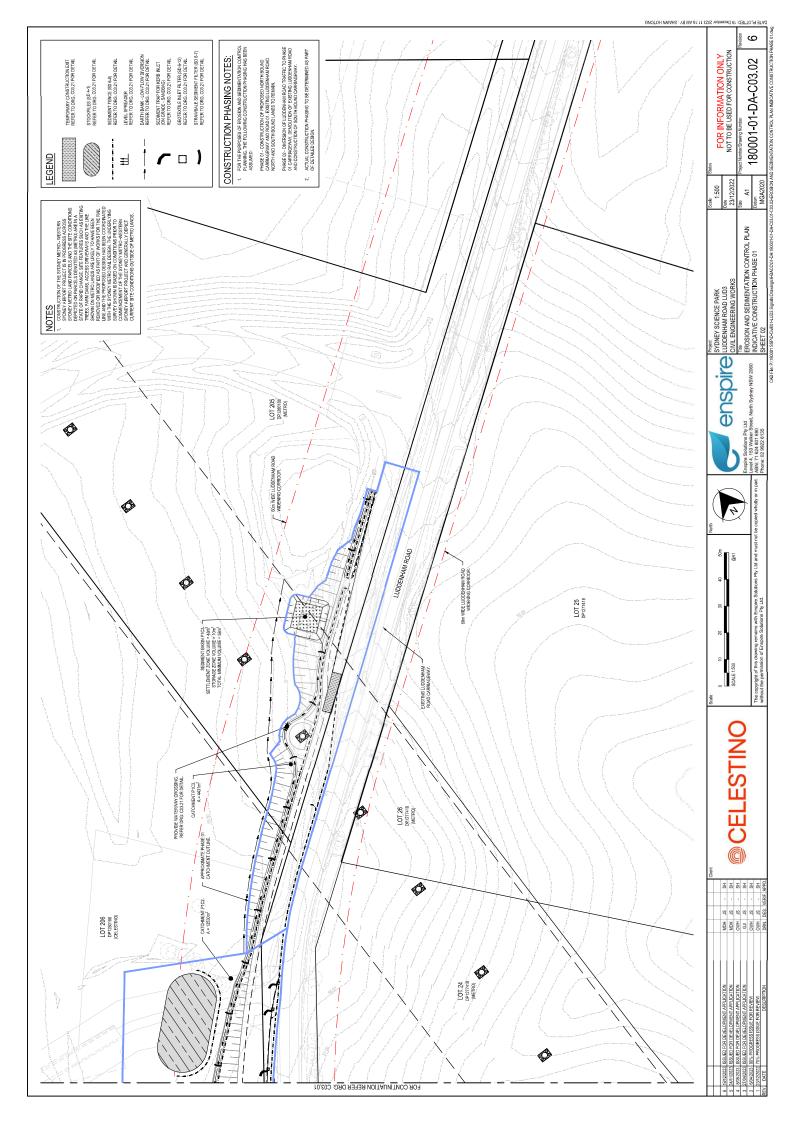


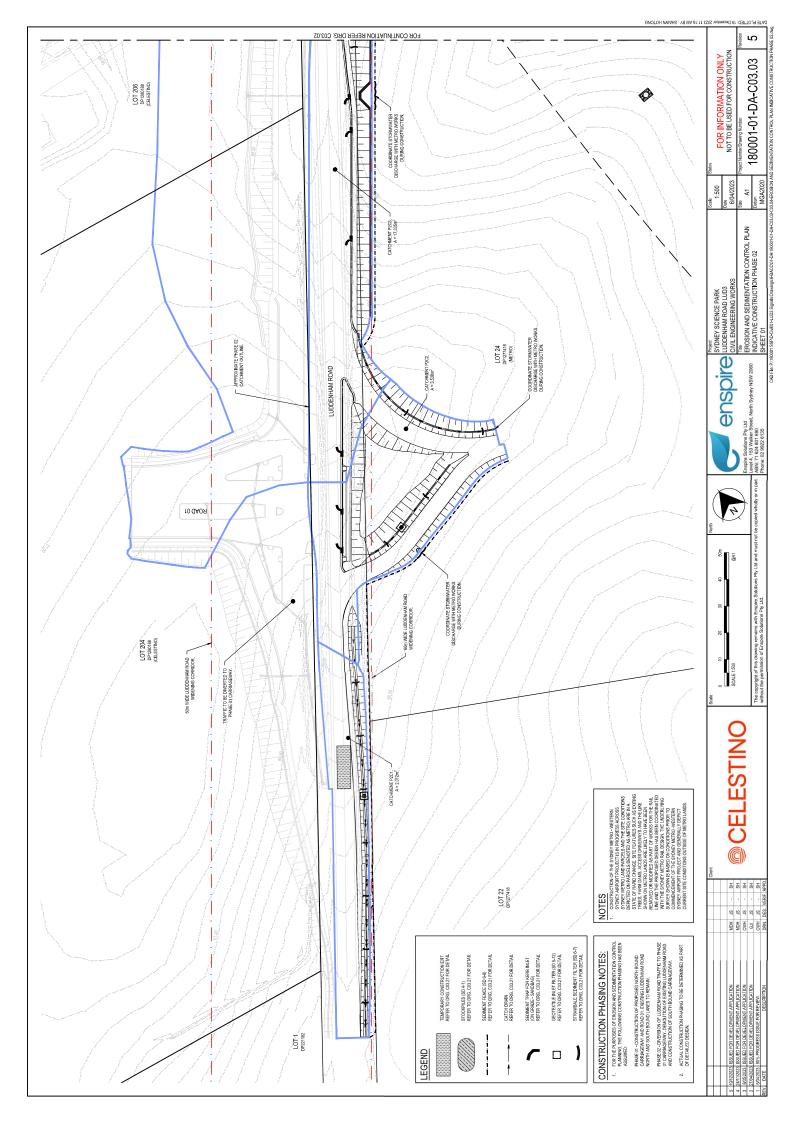


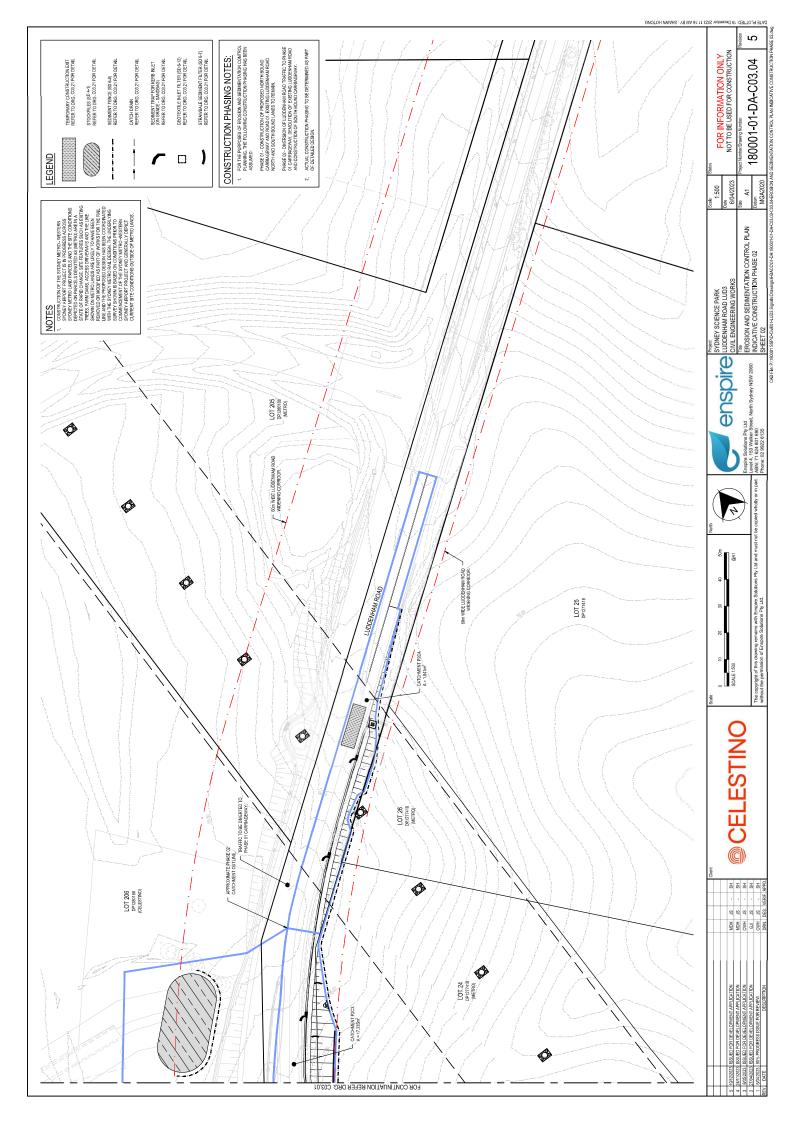


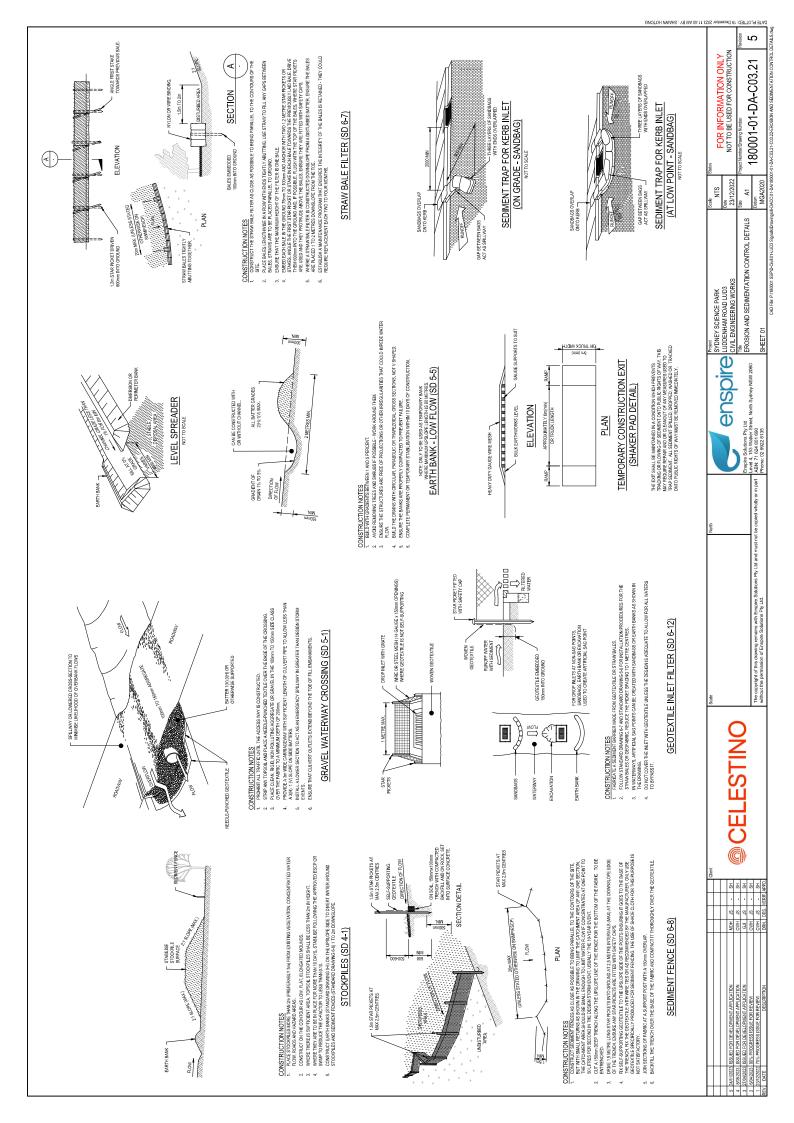










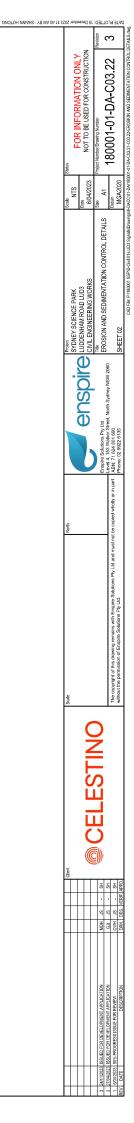


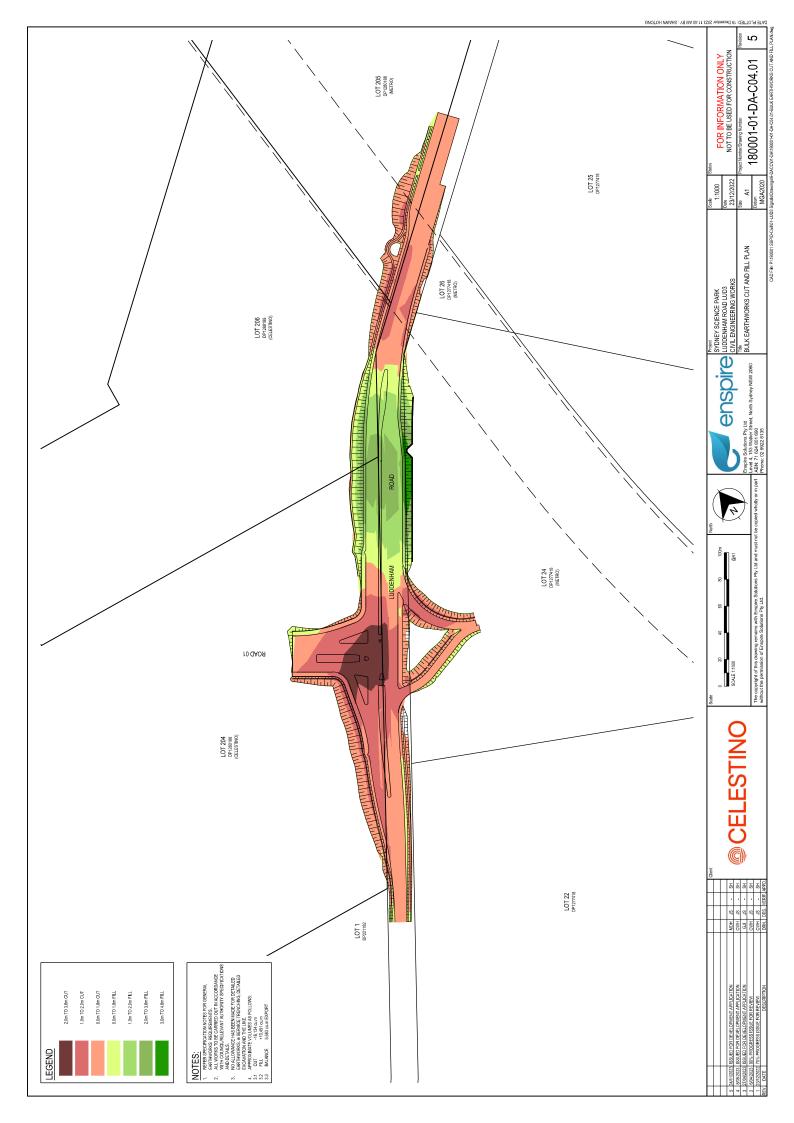
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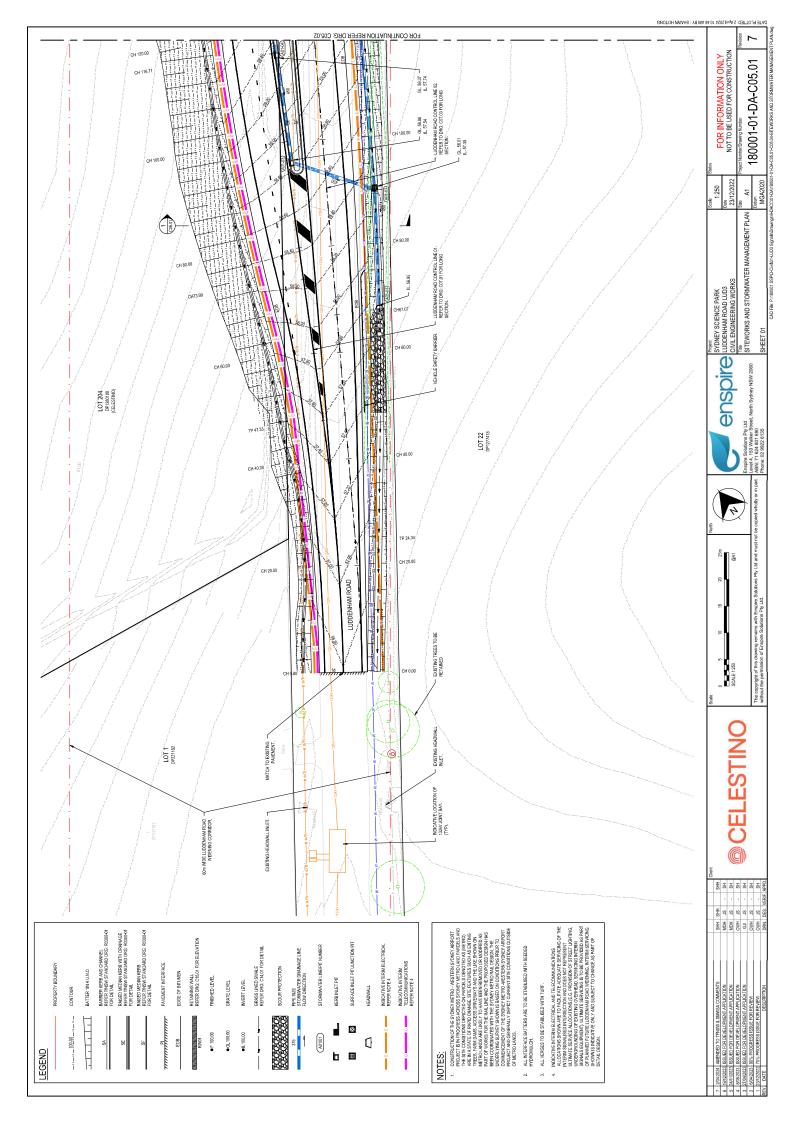
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	Soil loss (t/ha/yr)	100	87	58			-		Soil loss (thatyr)		61	87			
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	Soil loss (m ³ /ha/yr)	11	67	8			C	Conversion to cubic metres	Soil loss (m ³ /halyr)		47	67			Conversion to cubic metres
	Sediment basin storage (soil) volume (m ³)	6	7	~			s	See Sectors 6.3.4(i) for calculators	Sediment basin storage (soil) volume (m ³)		4	÷		0.	See Sections 6.3.4(i) for calculations
	Sediment basin setting (water) volume (m3)	5	8	89 k		1	-	See Sections 6.3.4(i) for calculations	Sediment basin setting (water) volume (m ³)		88	207	+		See Sections 6.3.4(i) for calculations
	Sediment basin total volume (m ³)	8	147	8			1		Sediment basin total volume (m ³)		98	218	-		
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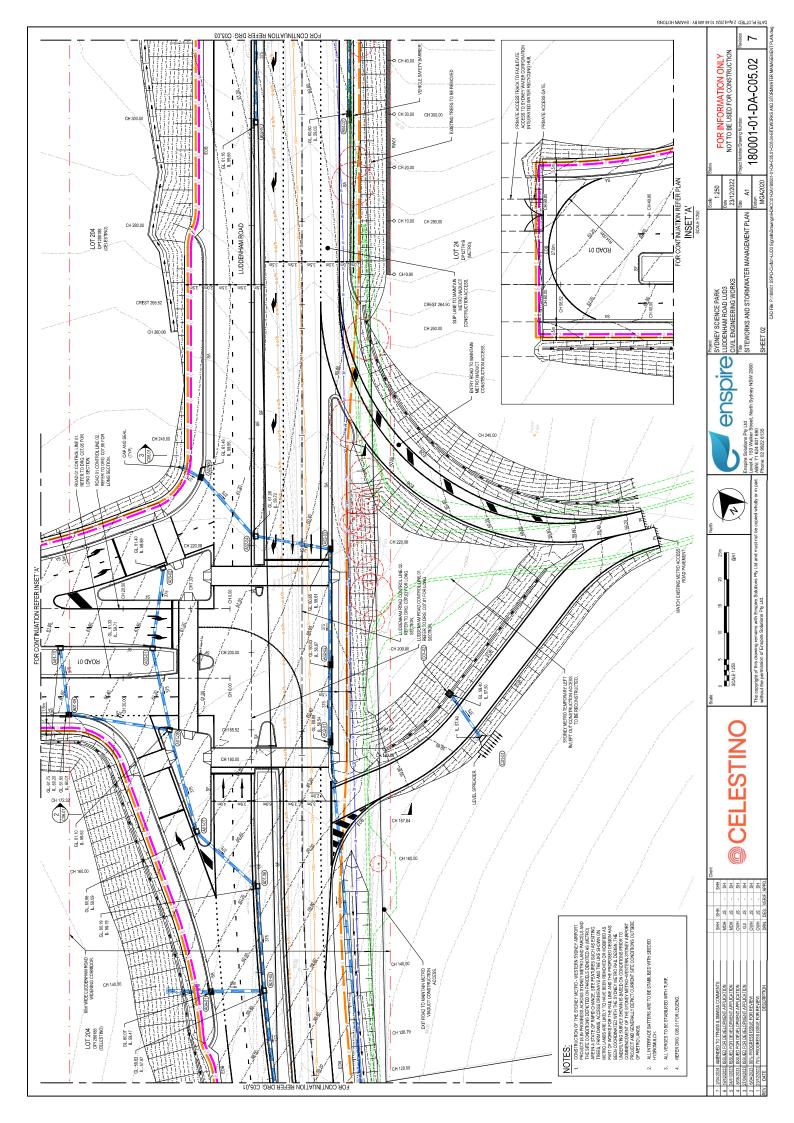
NB for sizing of Type C (coarse) sediment basins, see Worksheet 3 (if required).

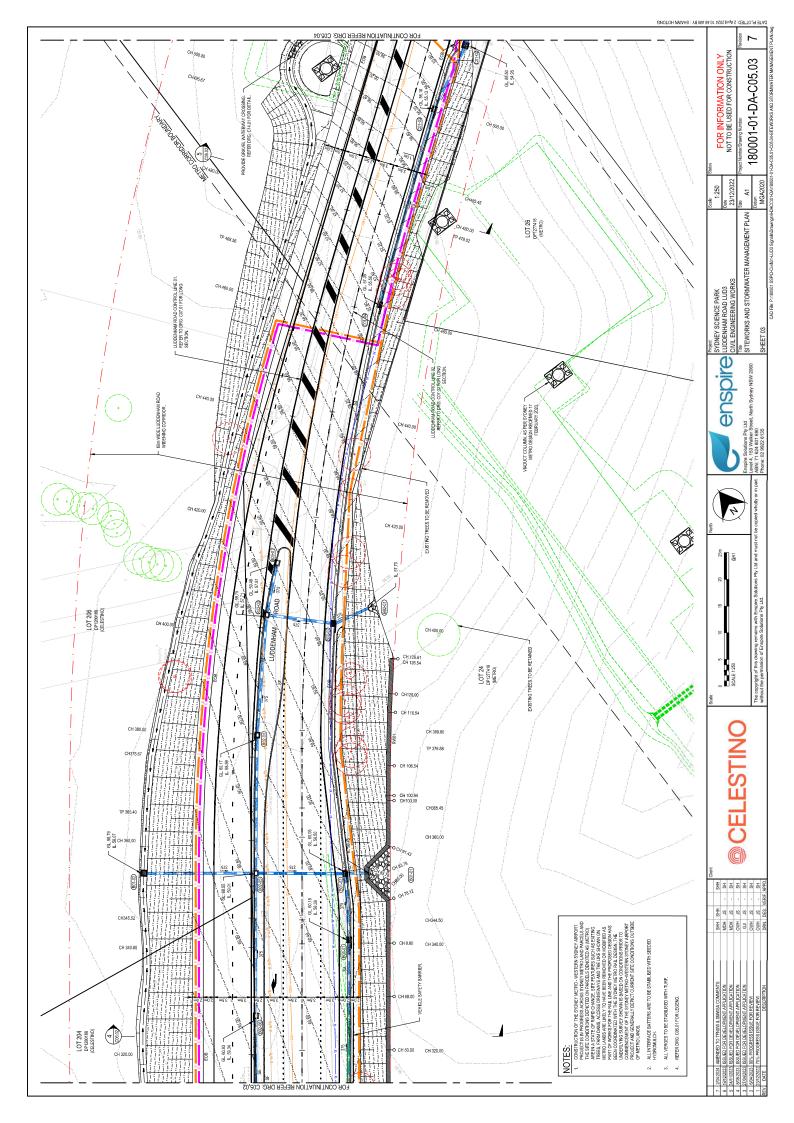
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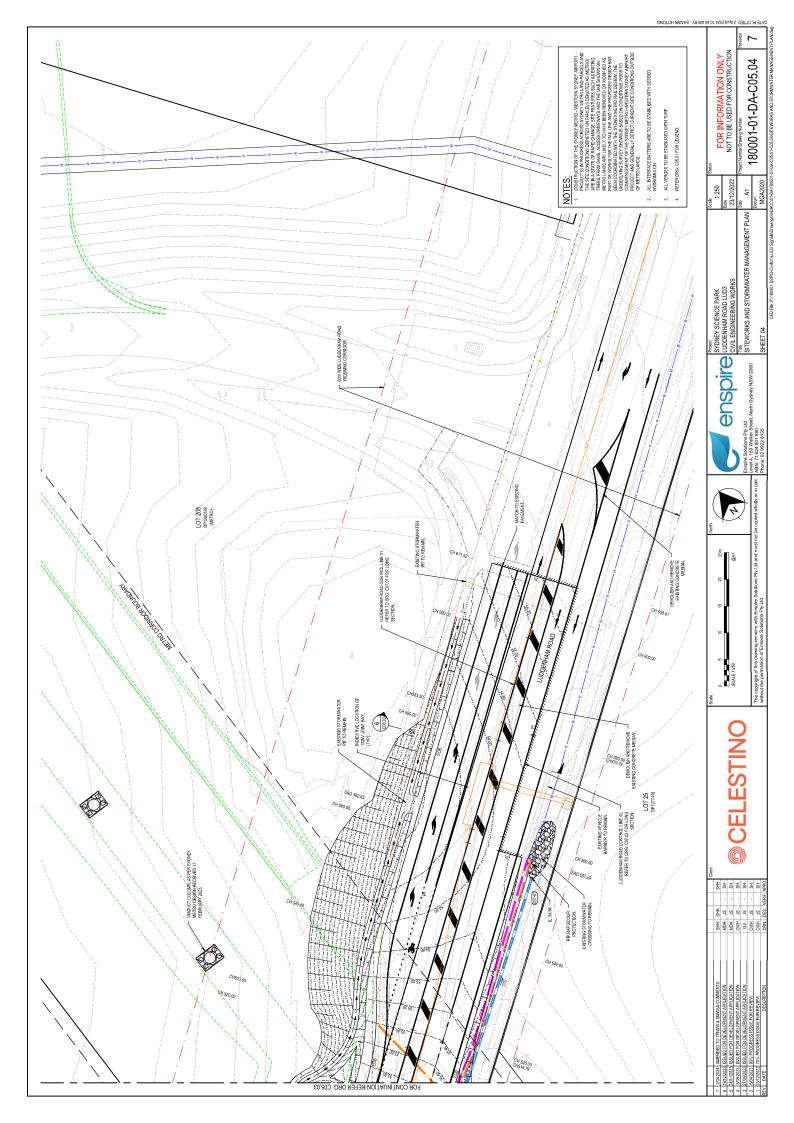


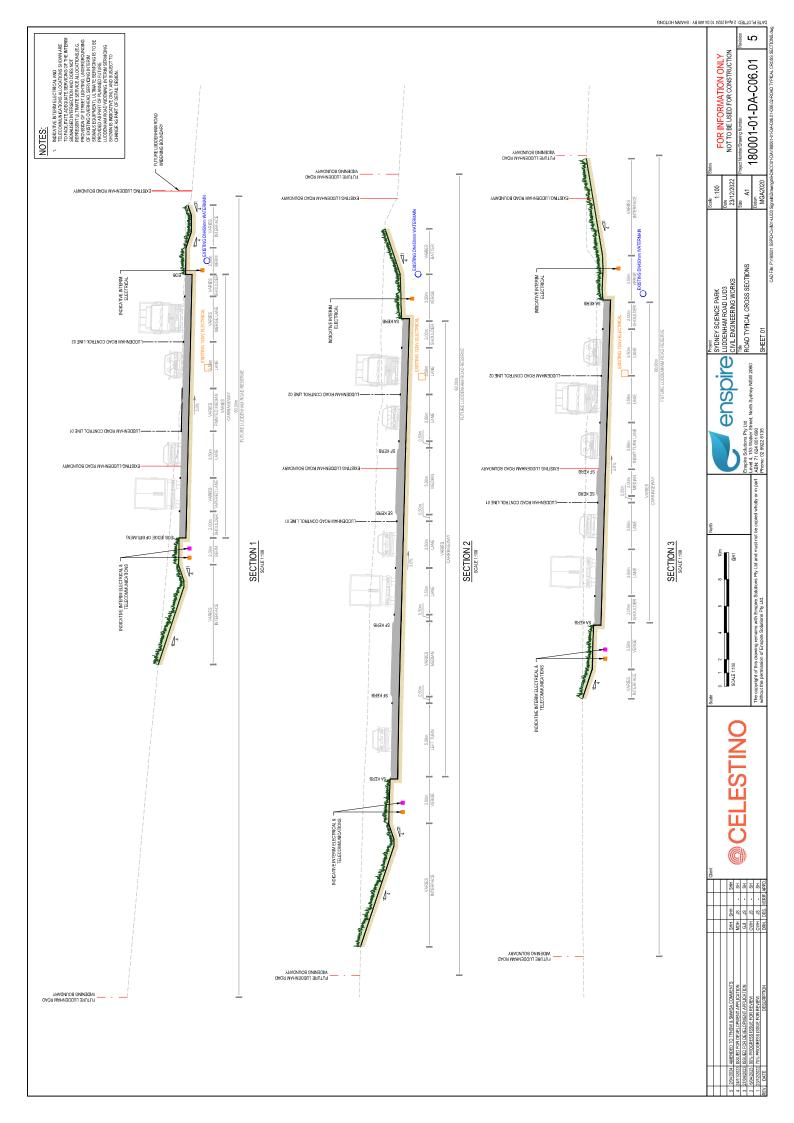


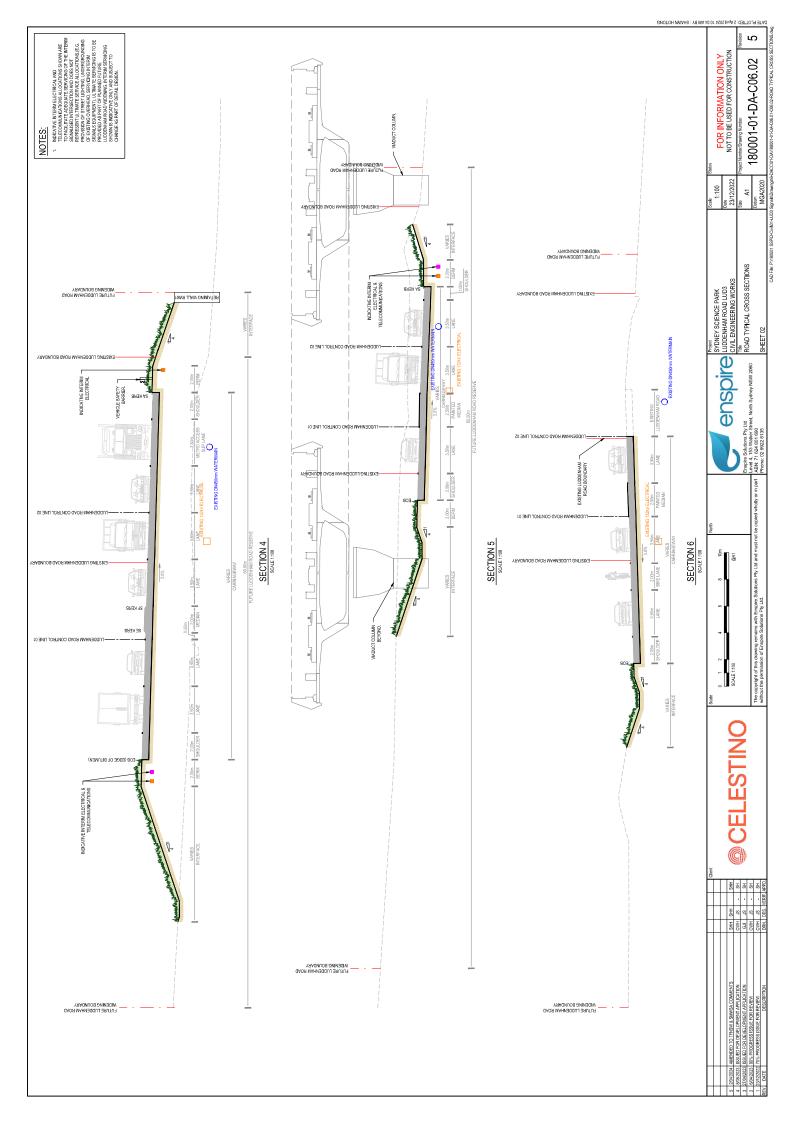


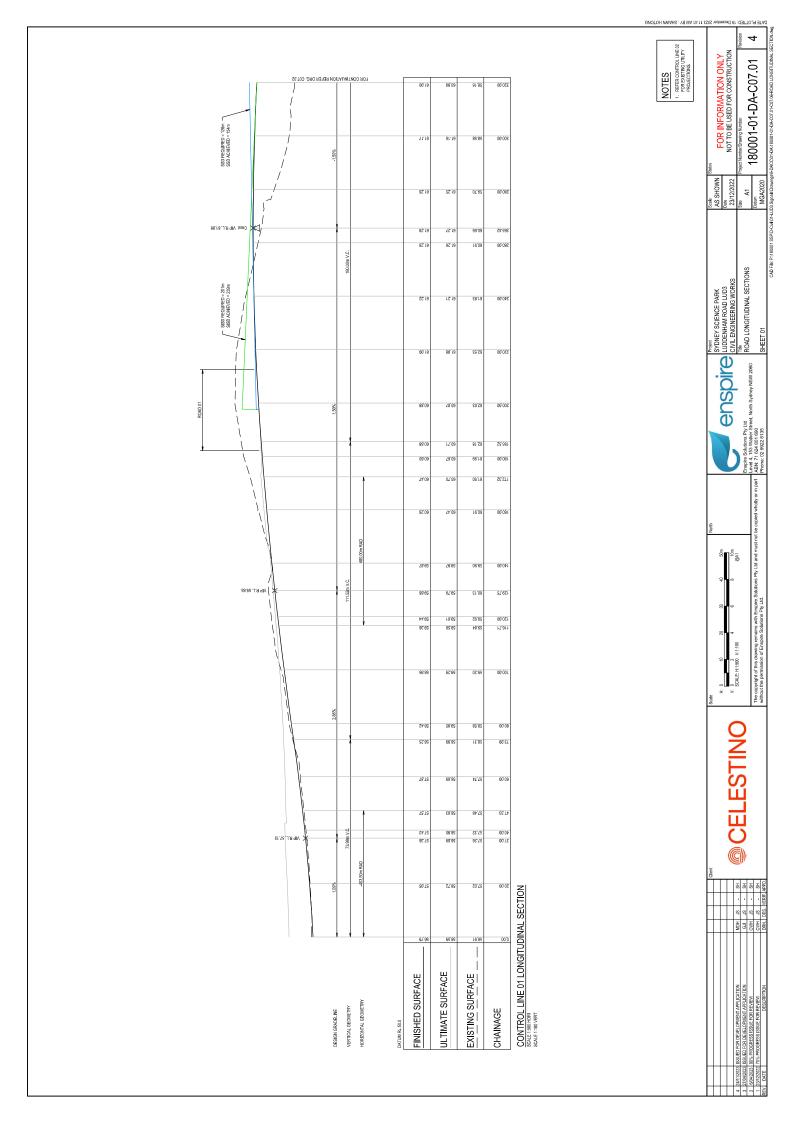


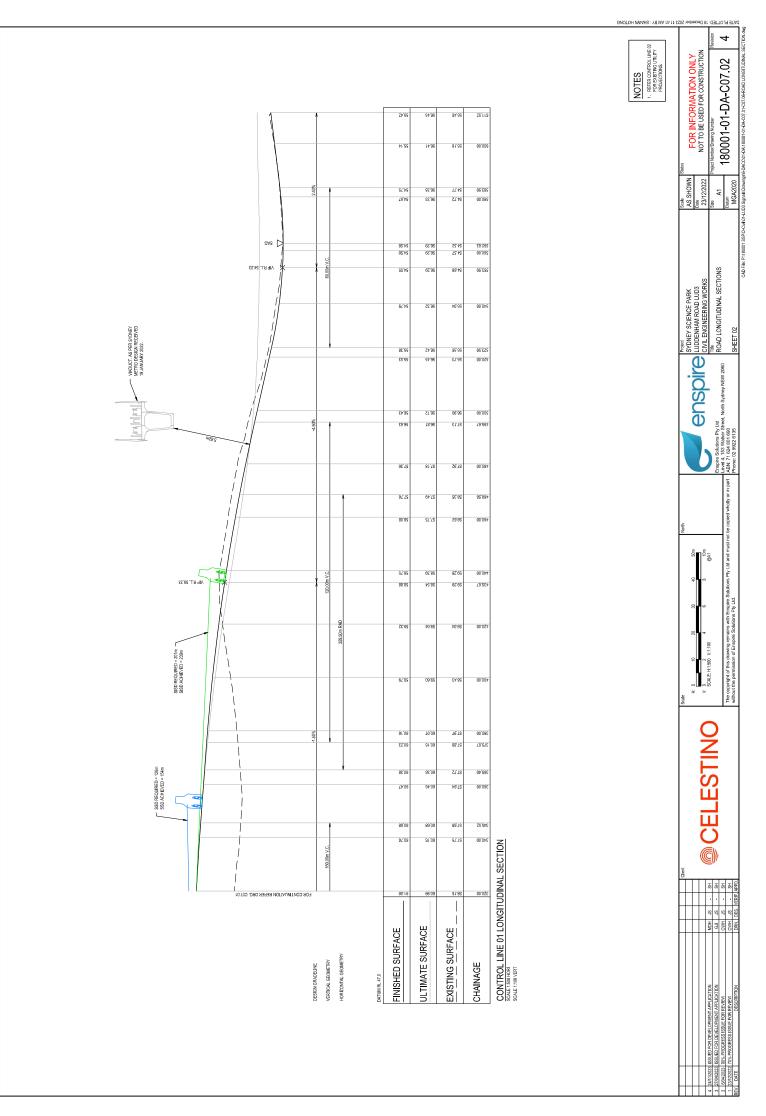


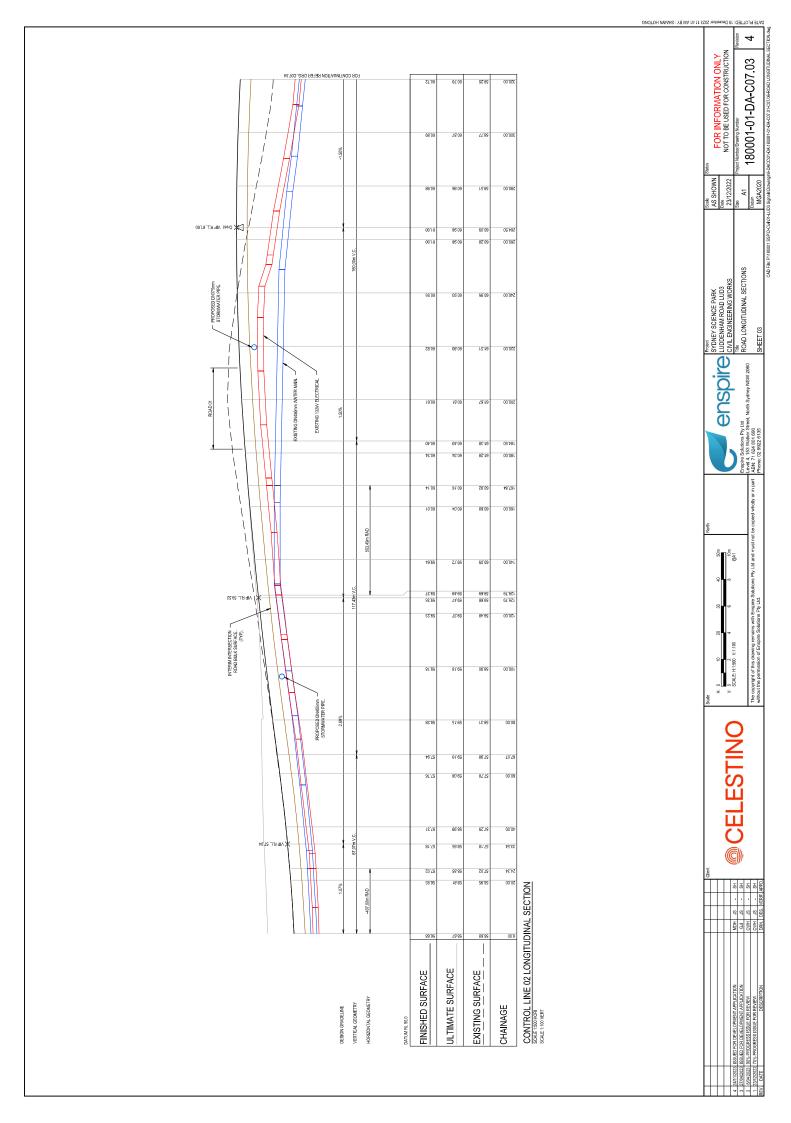


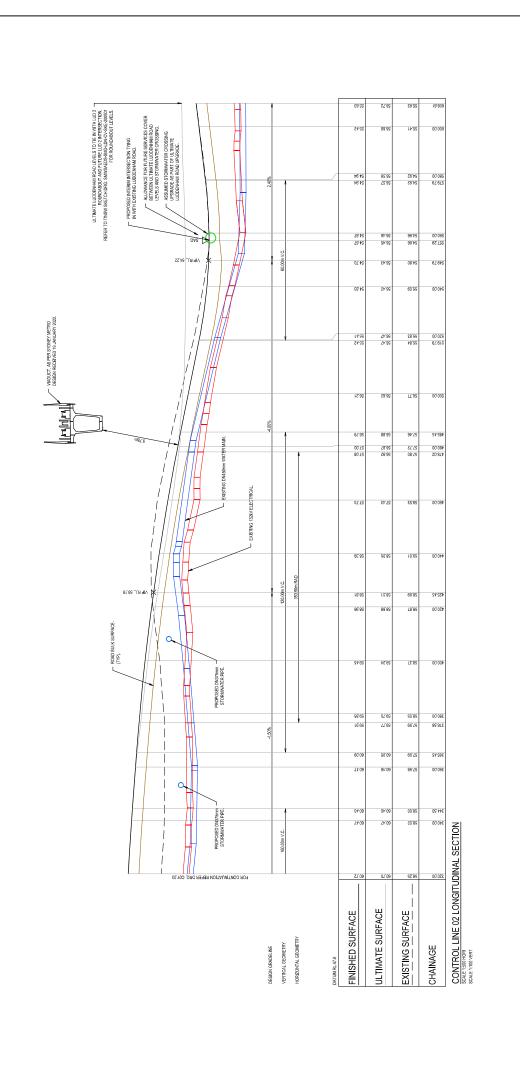


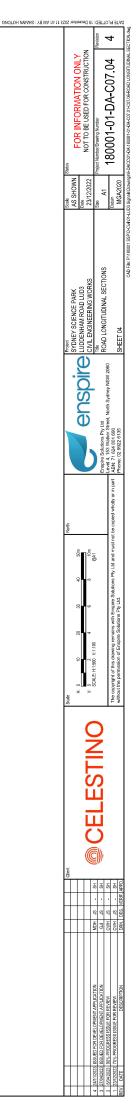


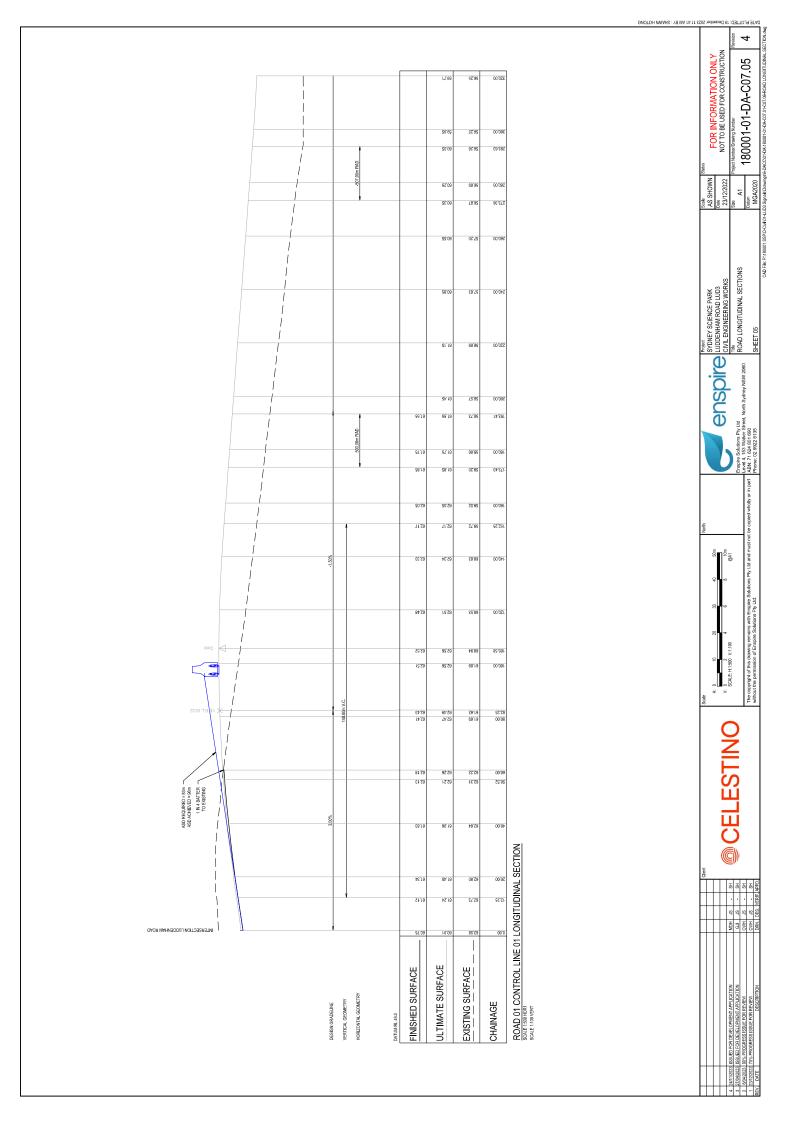


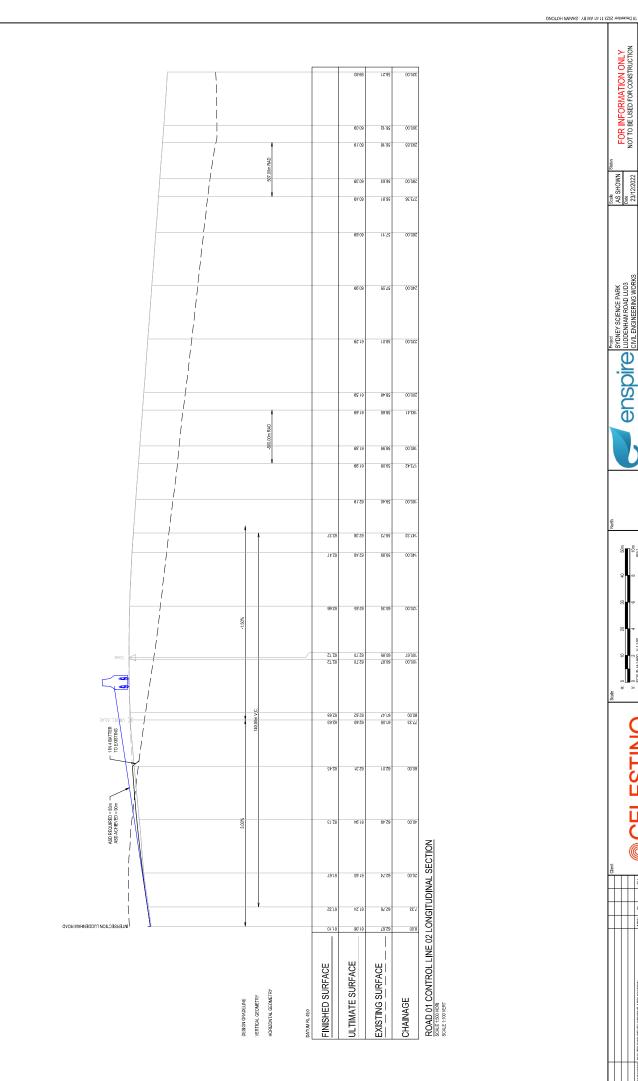


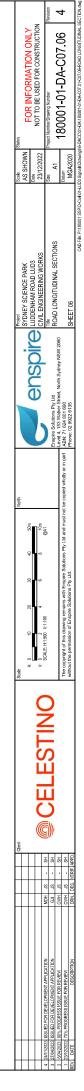




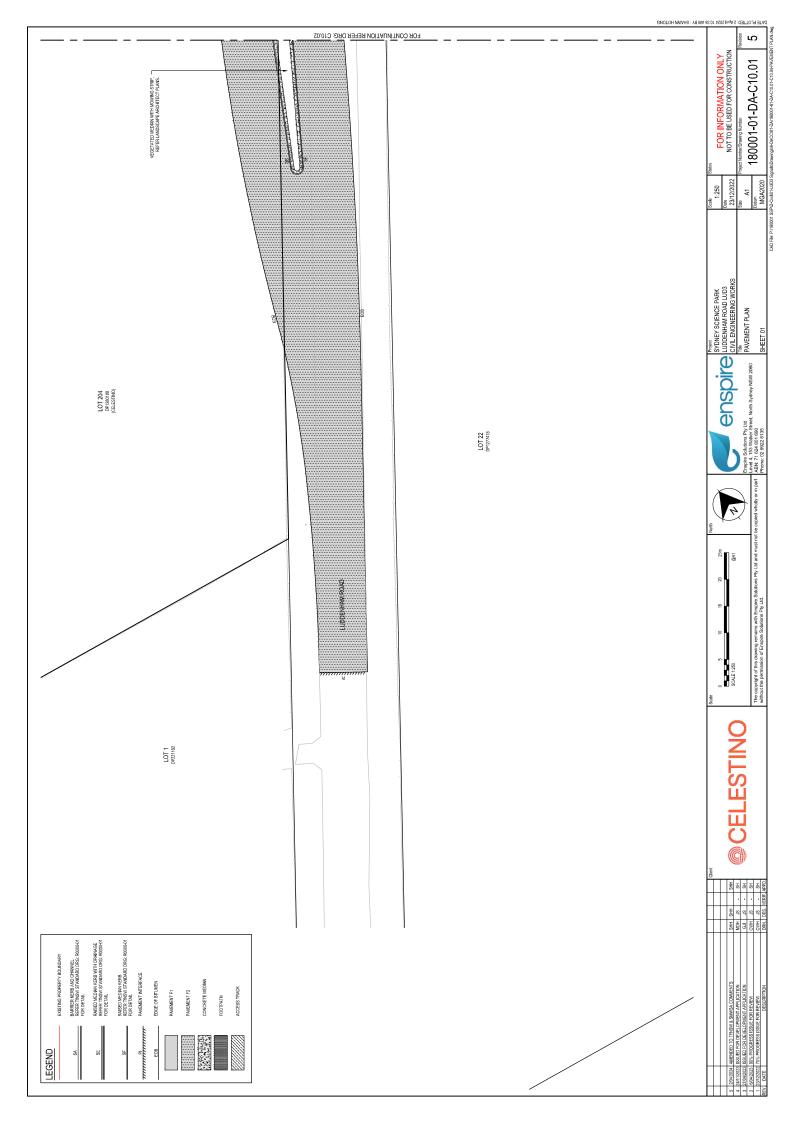


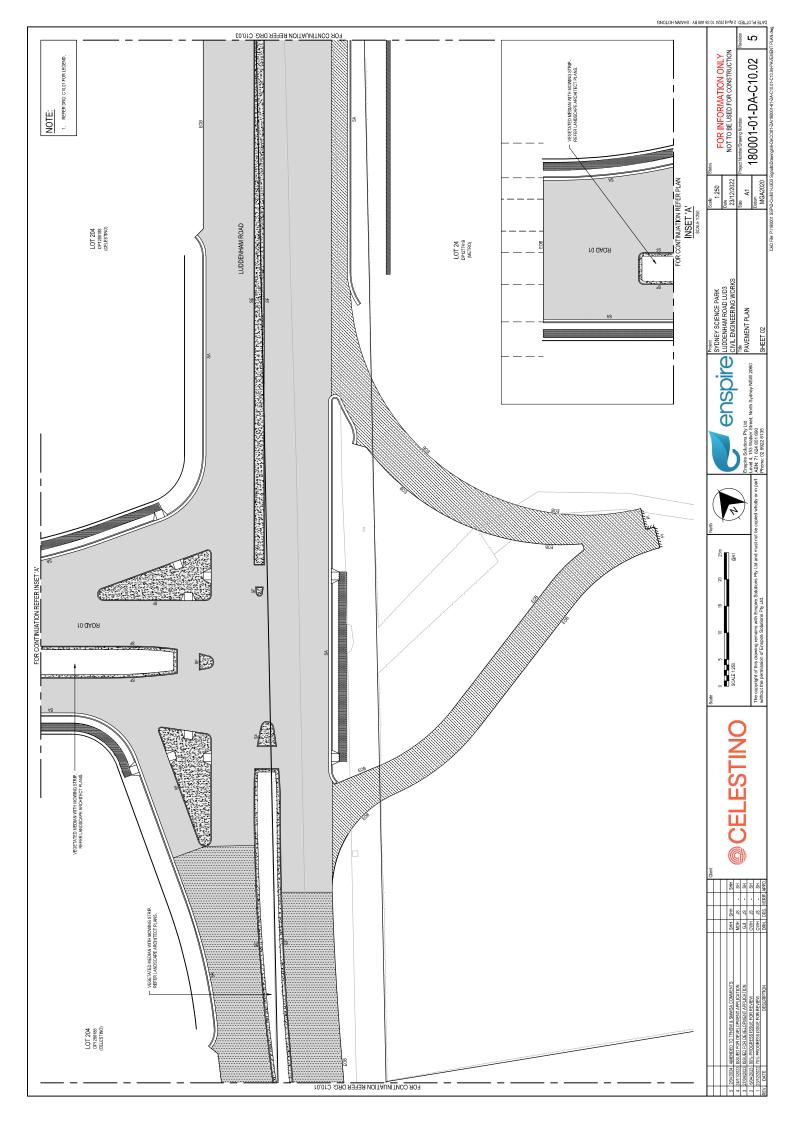


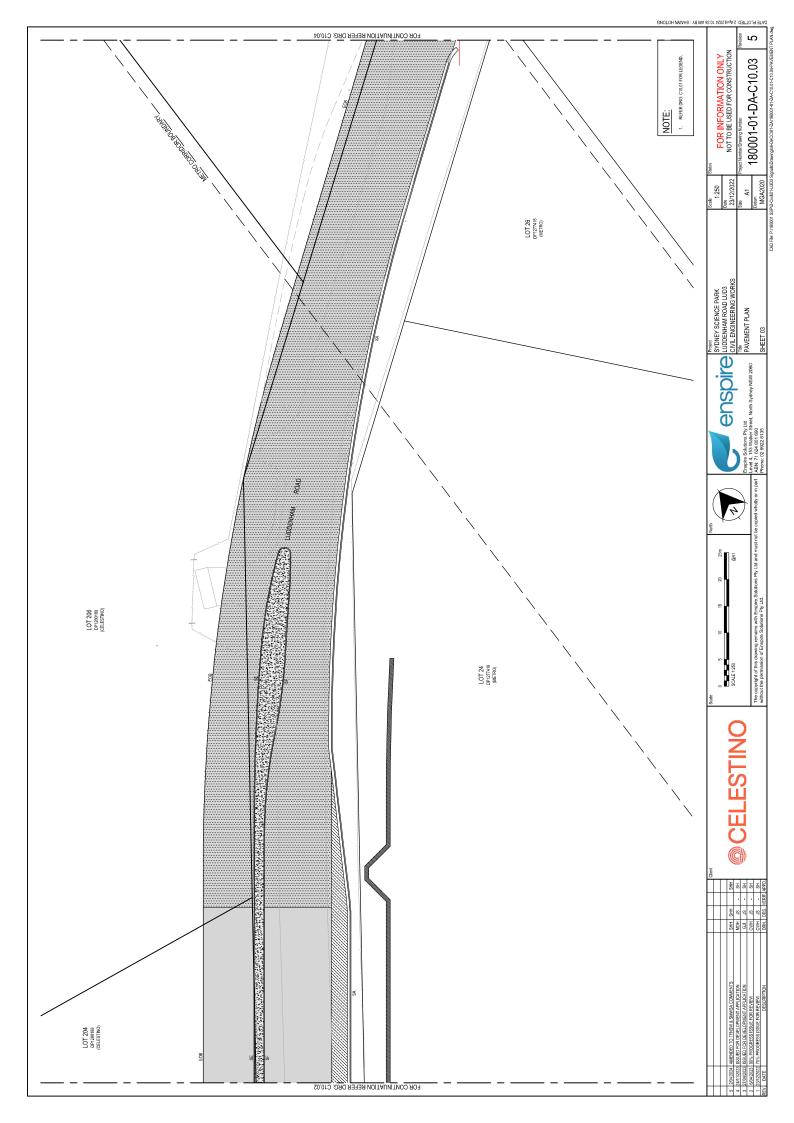


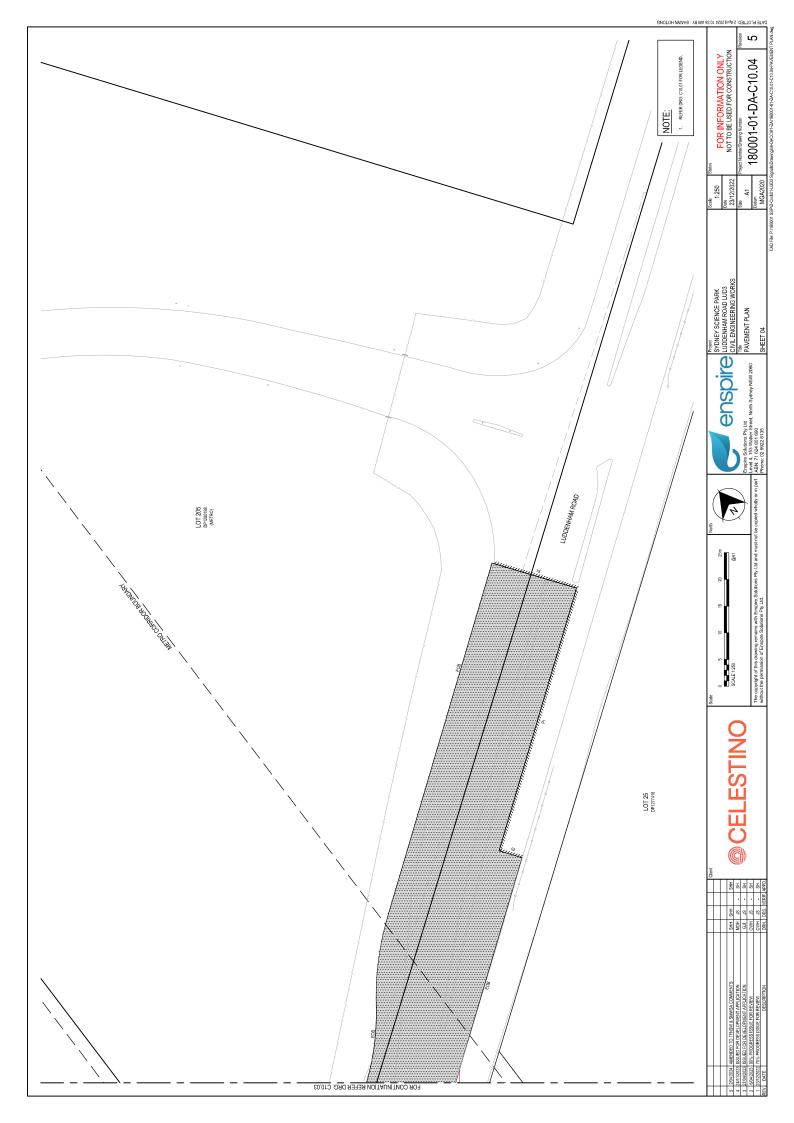


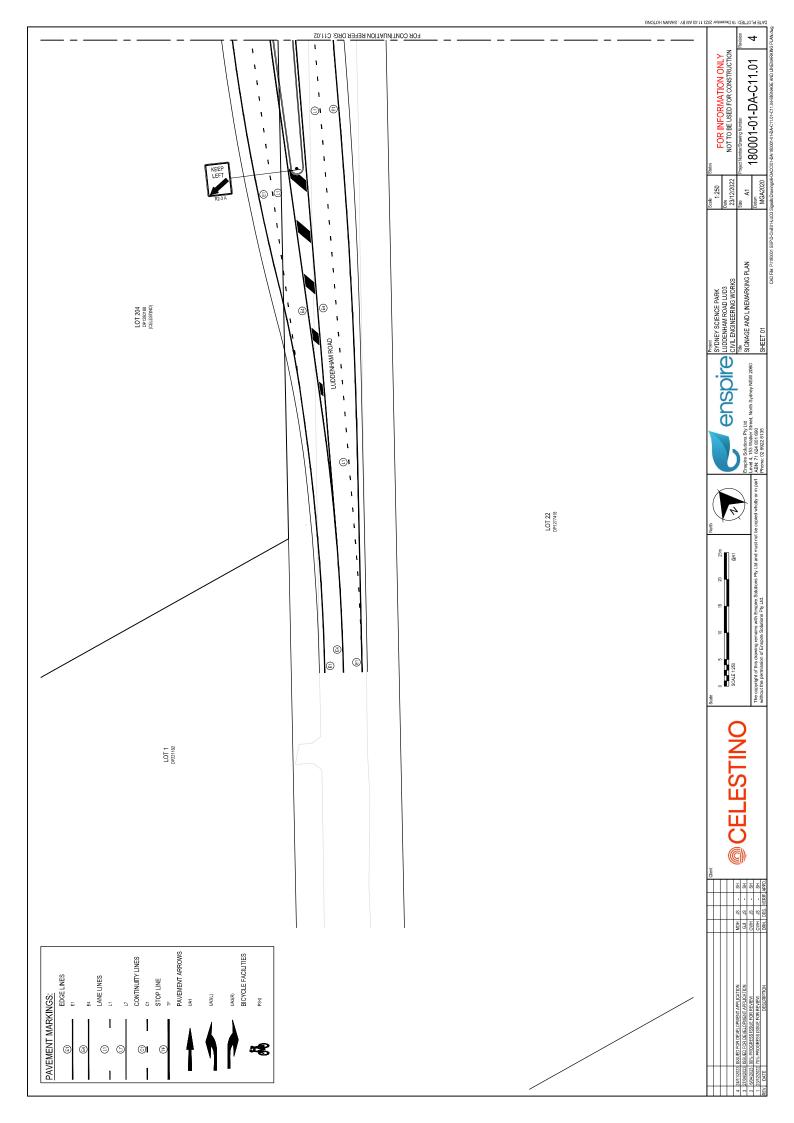
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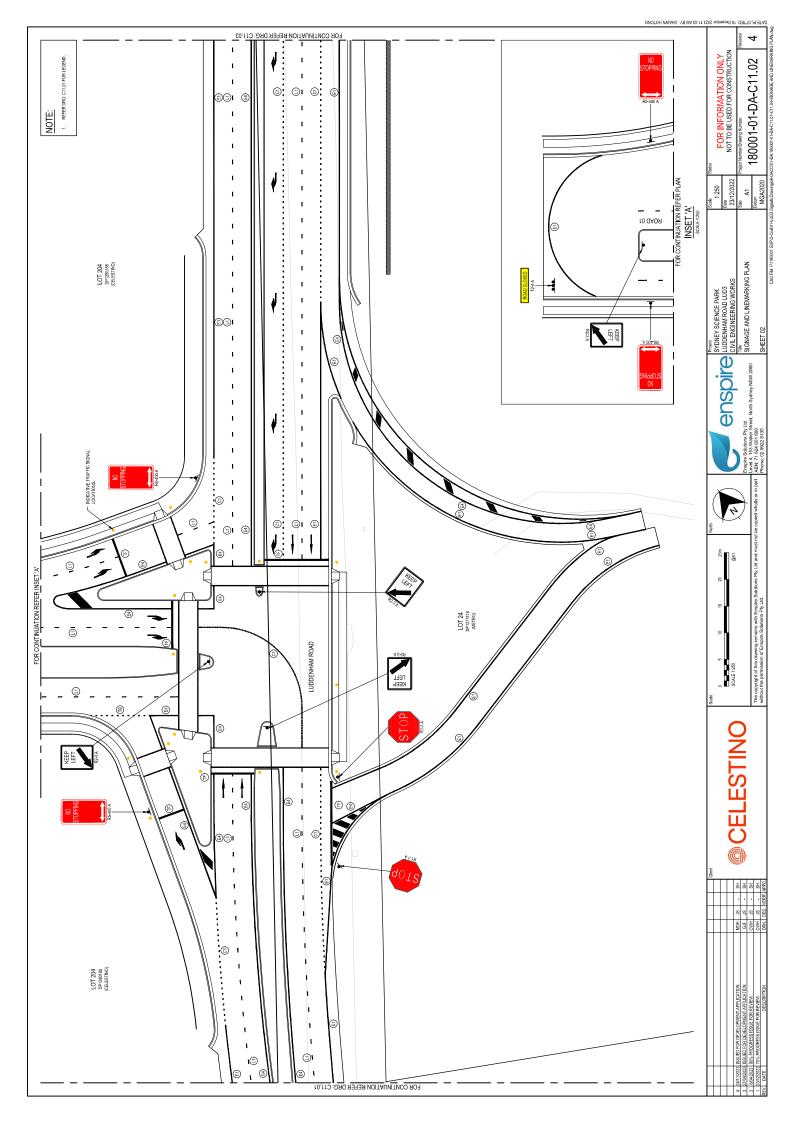


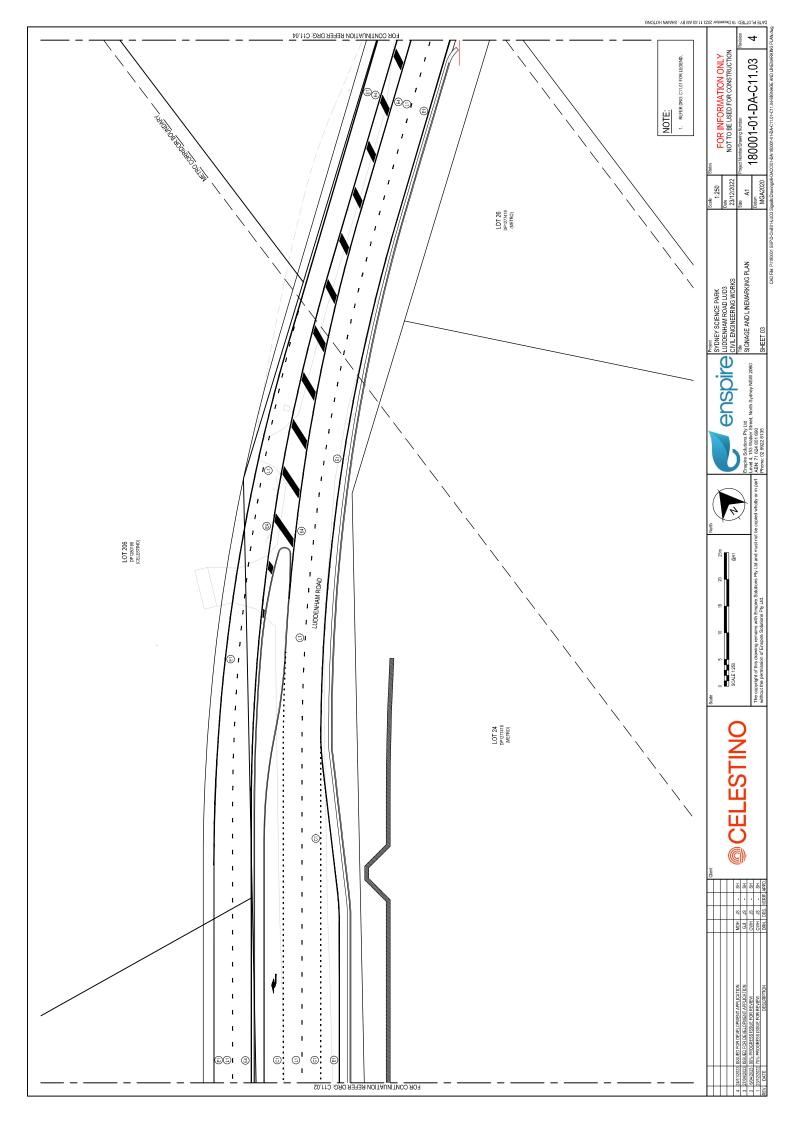


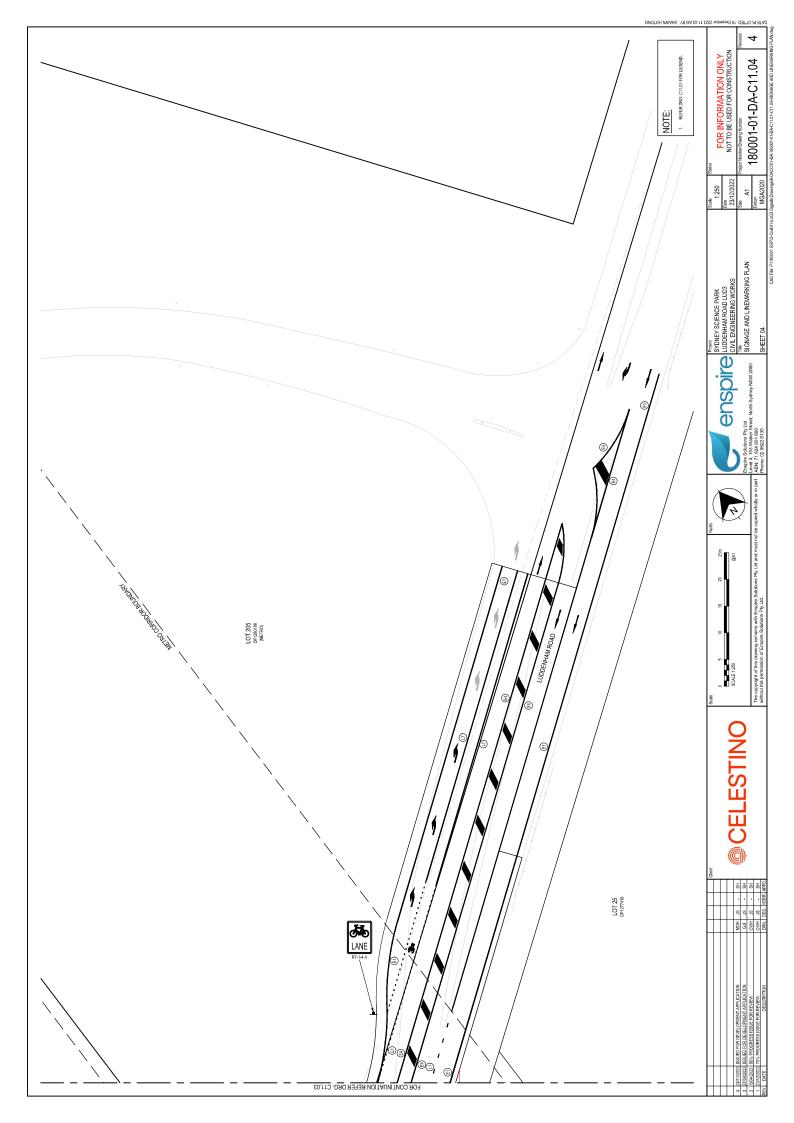


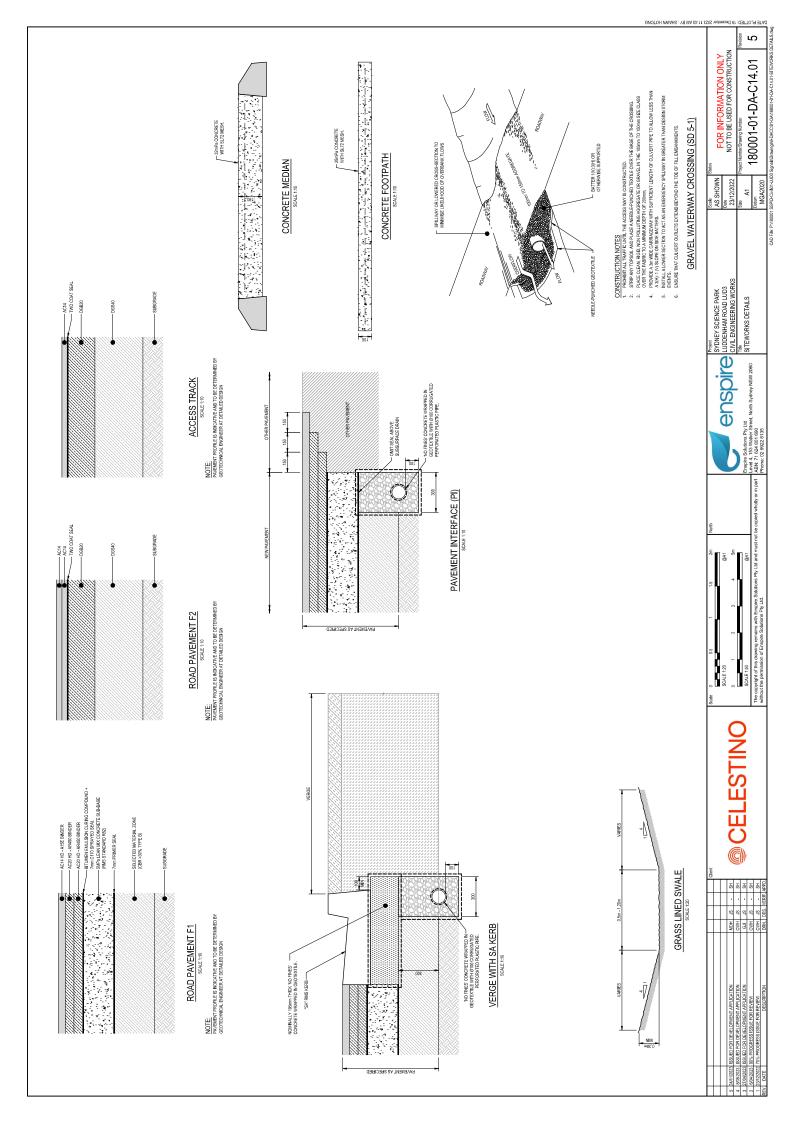


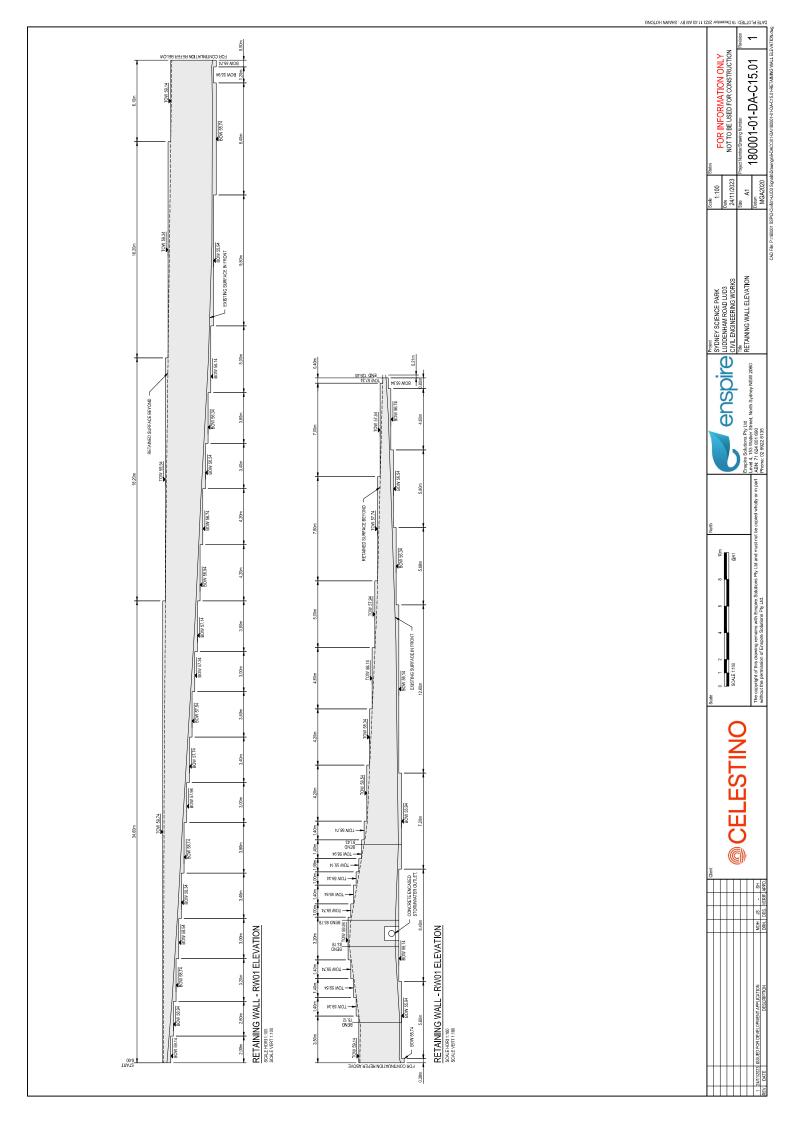


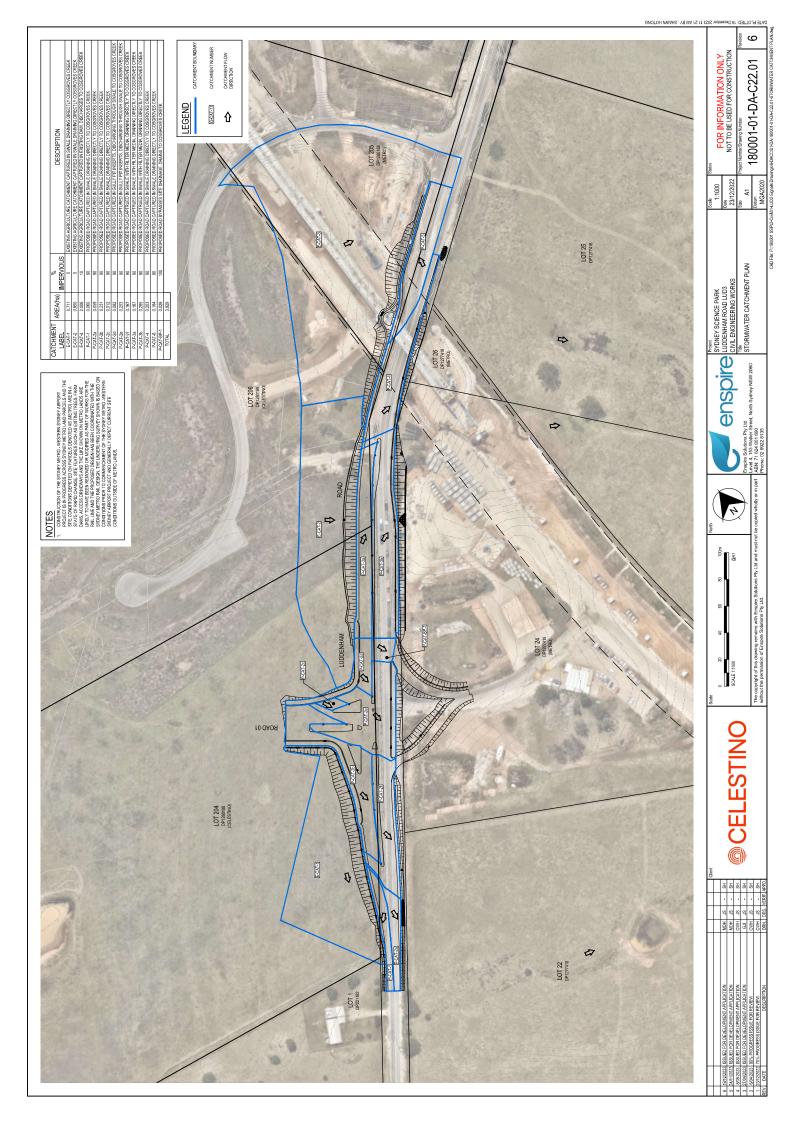


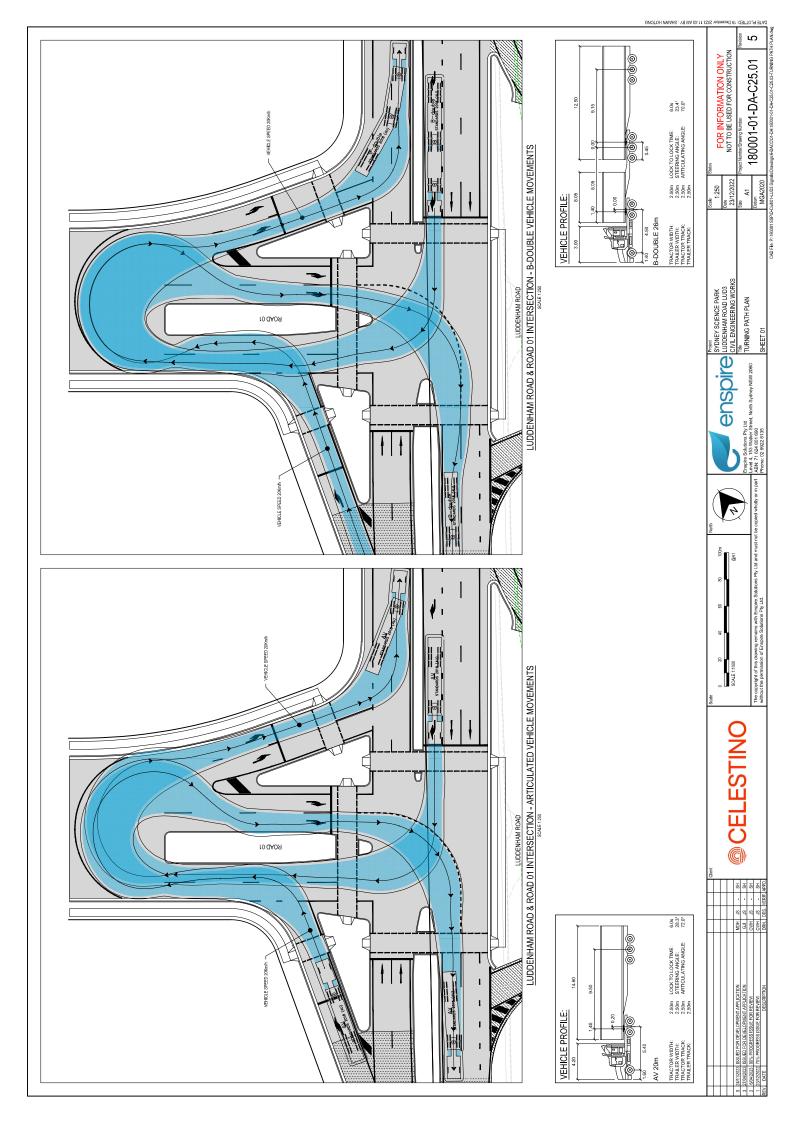


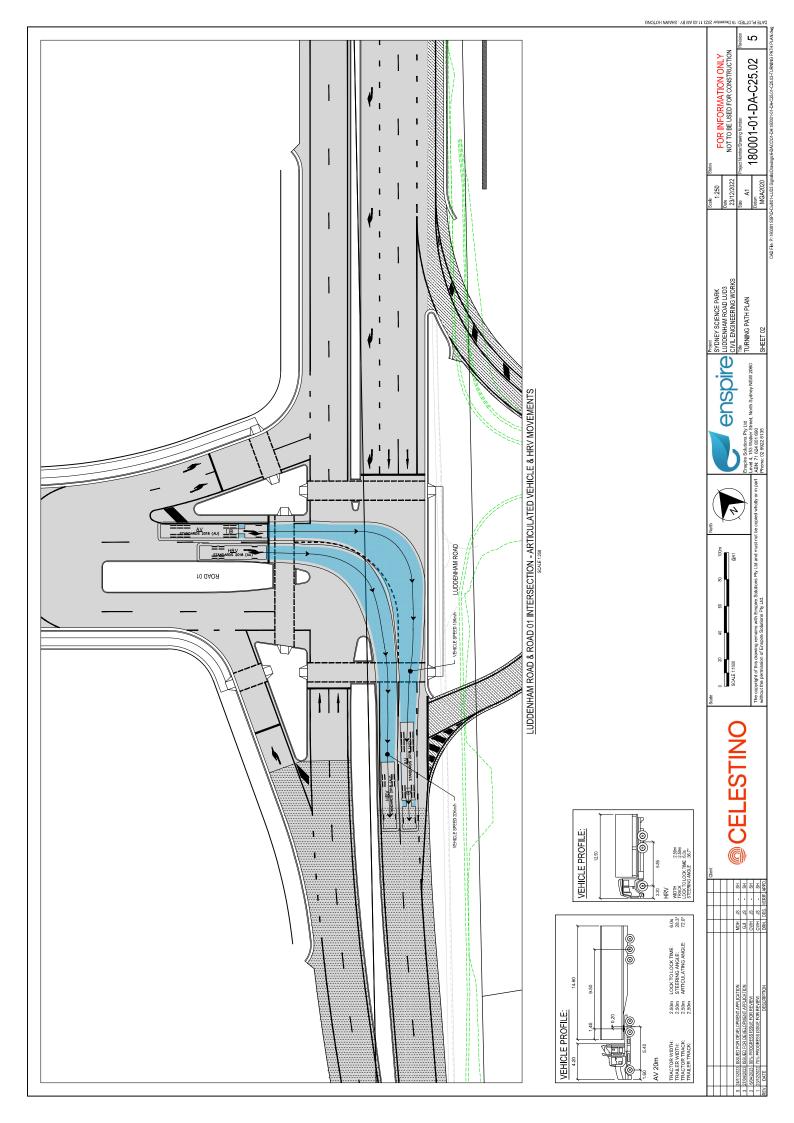


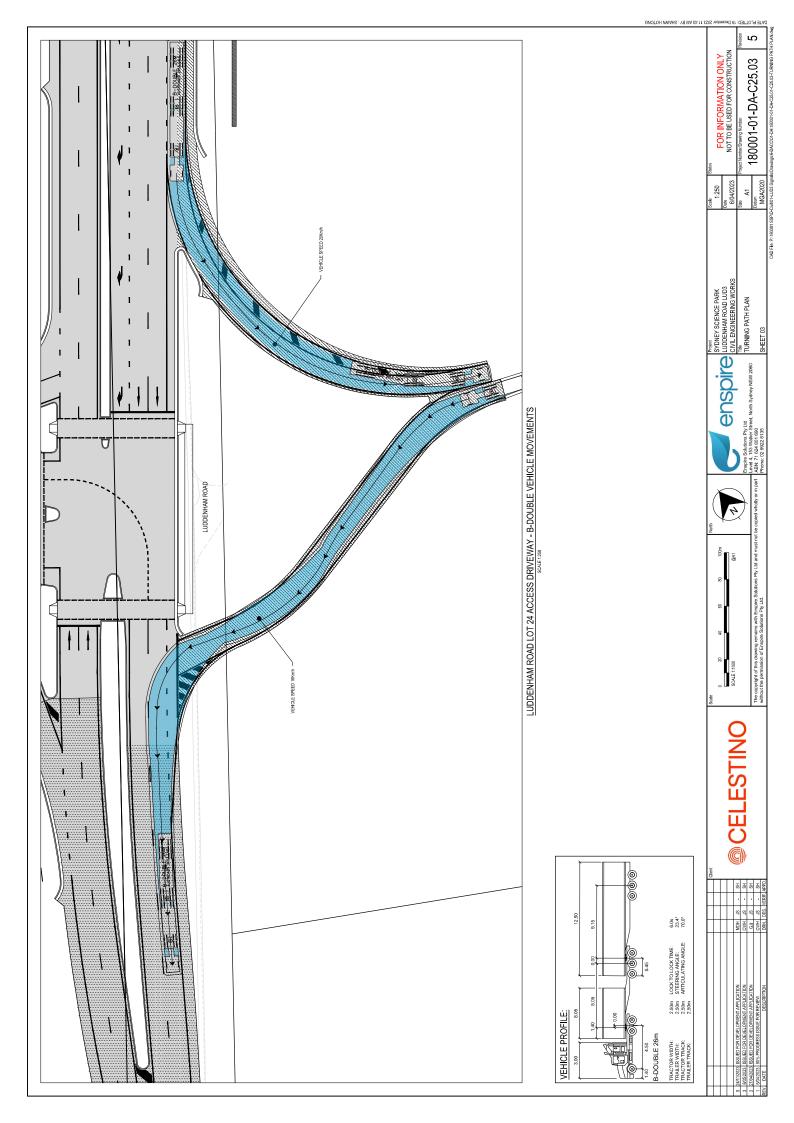












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LEVEL	MEASURE	CRITERIA
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2	UNLIKELY	NOT LIKELY TO OCCUR IN THE DEVELOPMENT'S LIFECYCLE FERIOD. A SMALL, BUT REMOTE CHANCE OF OCCURRENCE DUE TO CRCUMSTANCES / STIUNTOWS THAT COULD ARSE.
e	POSSIBLE	LIKELY TO OCCUR AT LEAST ONCE BUT NOT EXPECTED TO OCCUR MUCH MORE THAT THIS IN THE DEVELOPMENTS LIFECVCLE PERIOD.
4	LIKELY	LIKELY TO OCCUR MORE THAN ONCE IN THE DEVELOPMENT'S LIFECYCLE PERIOD BUT NOT AN EVERYDAY OCCURRENCE, PRECONDITIONS WILL ARISE AT TIMES THROUGHOUT THE PERIOD.
2	ALMOST CERTAIN	ALLIOST CERTAN INL OCCUR. CIRCUMSTANCES OR STILLYIONS ARE LIKELY TO ARRECTEN THROUGHOUT THE DERELORMENTS. ALLIOST CERTAN IL LEFENCIE FERRIO MACH PROVIDES THE OPPORTUNITY FOR CHYSTALLISNTION OF RISK EXPECT FREQUENT. TREGULAR COLORRENCES.

J	UALITATI	QUALITATIVE MEASURES OF IMPACT - CONSEQUENCE SEVERITY
LEVEL	MEASURE	CRITERIA
-	INSIGNIFICANT	INSIGNIFICANT NO NURRES, NO ENVIRONMENTAL IMPACT.
2	MINOR	FIRST AID, ENVIRONMENTAL RELEASE IMMEDIATELY CONTAINED.
e	MODERATE	MEDICAL TREATMENT; ENVIRONMENTAL RELEASE NOT MINEDIATELY CONTAINED WITH NO DETRIMENTAL EFFECTS.
4	MAJOR	LOST TIME AND/OR LONG-TERM INJURY/ILLINESS, ENVIRONMENTAL RELEASE NOT MIXEDIATELY CONTAINED WITH TOXIC EFFECTS.
ŝ	CATASTROPHIC	CATASTROPHIC FATALITY: RELEASE TO THE ENVIRONMENT WITH LONG TERM OR PERMANENT TOXIC EFFECTS.

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SINCE	MAJOR	(†)	HIGH	HOH	VERY HIGH	ИСПУ ИСН	VERY HIGH
SEQUE	MODERATE	(3)	MODERATE	MODERATE MODERATE	HIGH	HIGH	VERY HIGH
CONS	MNOR	(2)	LOW	LOW	MODERATE	HCH	VERY HIGH
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		PERSON RESPONSIBLE FOR CONTROLS	CONTRACTOR	CONTRACTOR	CONTRACTOR	DESIGNER	DESIGNER	DESIGNER CONTRACTOR	DESIGNER	CONTRACTOR	Status	Project Number/Drawing Number 180001-01-DA-C26 01
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Appendix B TfNSW Signals In Principal Approval

TfNSW



TRANSPORT



Site Details			
TCS Site #	Street 1	Street 2:	
XXXX	Luddenham Road	Sydney Science Park	
Street 3	Suburb	LGA Name	
Click or tap here to enter	Penrith	Penrith	
text.			
Maintenance Group	State Electoral Boundary -	UDB/Ref:	
Project Details			
Program	Region	3 Cities	
N/A	Greater Sydney	Western Parkland City	
Client	Client Contact	Contact Email	
Recommended Network Operations Team Leader	Signature		Date 02/05/2023
Print name: Tim Dewberry			
Comments: Click or tap here t	to enter text.		
Approved	Signature		Date
Senior Manager Network and Safety Services	D. Mus		02/05/2023

Comments: Click or tap here to enter text.

Disclaimer:

This form provides Agreement in Principle to the addition or alteration of Traffic Signals at the stated location. As such it has been determined that traffic signals are an appropriate form of time separated traffic control at the stated location. Please note that following the commencement of the detailed design review unforeseen constraints may be identified which significantly affect the delivery of the project agreed to in principal by this form. This includes, but is not limited to, utility works, land ownership, property acquisition, and drainage.

Under normal circumstances this Agreement in Principle expires after the latter of:

- 5 years after the date of the signatures provided above;
- 5 years after the Notice of the Determination for a Development Application from a Consent Authority.

In extenuating circumstances, such as where traffic volumes, land use or network changes have substantially altered the road environment, Roads and Maritime reserves the right to withdraw this Agreement in Principle.



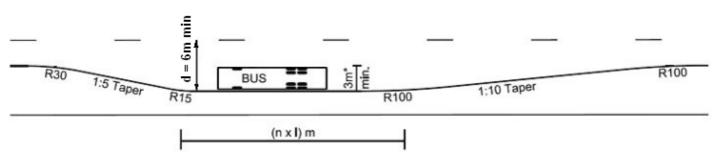
Appendix C Geometric Design Details

Enspire Solutions



4.12.3 Bus stops - rural

The approved layout for rural indented bus bays is shown below:



* Width (including clearances) may need to be increased where large/wide buses are involved n = number of buses using stop simultaneously I = length of bus (including clearances if desired)

d = either centreline of road, designated centreline or first lane line

5 Sight distance

5.2.2 Driver reaction time

Roads and Maritime practice is to use the following driver reaction times:

Reaction Time (s)	Design Speed (km/h)
2.5	≥ 110
2.0	100
1.5	≤ 90

Table 5.2: Driver reaction times

Note: Higher reaction times should be considered where local conditions warrant.

5.2.3 Longitudinal deceleration

Roads and Maritime uses a coefficient of deceleration of 0.36 for cars on sealed roads.

The tabled value of coefficient of deceleration for buses ensures passenger comfort when decelerating on the approach to a bus stop. This should be considered when designing bus specific facilities.

5.3.2 Truck stopping sight distance

Roads and Maritime does not use truck stopping sight distance as a normal design parameter. Truck stopping sight distance should be checked in approach to truck related facilities (such as inspection bays and weigh bridges), assuming the car / truck speed relationship shown in Table 3.5.

7 Horizontal alignment

7.5.1 Compound curves

In Roads and Maritime practice the desirable ratio of the larger radius to the smaller radius should not exceed 1:0.75. However, in low speed designs, where compound curves with radii less than 1000m are unavoidable, the larger radius to the smaller should not exceed 1:0.5. For high speed design, the design speed criteria and not curve ratios should be satisfied.

Vehicles and Turning Path Templates (2013)

7.6 Side friction and minimum curve size

Roads and Maritime uses the desirable maximum values of side friction for cars as the normal design parameter for side friction.

7.8 Curves with adverse crossfall

Roads and Maritime does not use the values shown in Table 7.12: Minimum radii with adverse crossfall for existing urban roads. Absolute maximum value for trucks

7.9 Pavement widening on horizontal curves

Roads and Maritime accepts the application of independently widening lanes or widening evenly across all lanes. Existing and/or proposed traffic composition and lane usage should be considered.

8 Vertical alignment

8.6.7 Minimum length of vertical curves

Roads and Maritime does not use the values shown in Table 8.11: Minimum length vertical curves for reconstruction.

Α Extended design domain (EDD) for geometric road design

A.5 **Pavement widening**

Where normal design domain values for lane widening on curves cannot be achieved, lane widening can be calculated using the following formula. The need for lane widening ceases when widening per lane is less than 0.2 metres.

$$W = \left(\sqrt{R^2 + A(2L + A)} - \sqrt{R^2 - \sum L_i^2}\right) x \left(1 - e^{\frac{-0.015 \times D \times R}{\sqrt{\sum L_i^2}}}\right) + W_V + C_I$$

Where:

Design vehicle	∑Li²	L	А	Wv
Passenger vehicle (5.2m)	9.3025	3.05	0.95	1.94
Service vehicle (8.8m)	25	5	1.5	2.5
Single unit truck / bus (12.5m)	46.9225	6.85	2.2	2.5
Long rigid bus (14.5m)	70.56	8.4	2.6	2.5
Articulated bus (19m)	61.21	5.5	2.6	2.5
Prime move and semi-trailer (19m)	118.3	5.3	1.6	2.5
Prime move and semi-trailer (25m)	222.21	5.4	1.6	3.0
B-double (25m)	169.81	4	1	2.5
B-double (26m)	168.775	4.5	1.4	2.5
A double (Type I) (36.2m)	228.9	5.5	1.6	2.5
B triple (35.4m)	245.99	5	1.5	2.5
A triple	333.29	6	1.7	2.5
Note: The design vehicles listed in the tal	ble are those list	ed in Au	stroads I	Design

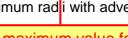
- = Widened lane width (m)
- Wv = Width of vehicle
- R = Radius (m)

W

e

- = Exponential mathematical constant "e"
- D = Degree of curvature (degrees)
- = Lateral clearance C_{I} (m)
- = Wheelbase of L single unit or prime mover (m)

= Front overhang of Α inner lane vehicle (m)



checked for safety in design.

		Based on approach sight distance for a car ⁽¹⁾ $h_1 = 1.1, h_2 = 0, d = 0.36^{(2)}$					
Design speed (km/h)	<i>R</i> ₇ = 1	$R_T = 1.5 \mathrm{sec}^{(3)}$		<i>R</i> ₇ = 2.0 sec		<i>R</i> ₇ = 2.5 sec	
	ASD (m)	К	ASD (m)	K	ASD (m)	К	
40 Road	01 34	5.3	40	7.2	_	-	
50	48	10.5	55	13.8	-	-	
60	64	18.8	73	24.0	-	-	
70	83	31.1	92	38.9	-	-	
80	103	48.5	114	59.5	-	-	
90	126	72.3	139	87.3	151	104	
100	151	104	165	124	179	146	
110	-	-	193	171	209	198	
120	m Deed	-	224	229	241	264	
110 ¹ Luddenha		-	257	301	275	344	

 Table 3.1:
 Approach sight distance (ASD) and corresponding minimum crest vertical curve size for sealed roads (S < L)</th>

Truck stopping capability provided by the minimum crest curve size⁽⁴⁾

 $h_1 = 2.4$ m, $h_2 = 0$ m, d = 0.22

1 If the average grade over the braking length is not zero, calculate the approach sight distance (ASD) values using the correction factors in Table 3.4 (or use Equation 1) by applying the average grade over the braking length.

2 In constrained locations (typically lower volume roads, less important roads, mountainous roads, lower speed urban roads and tunnels), a coefficient of deceleration of 0.46 may be used. For any horizontal curve with a side friction factor greater than the desirable maximum value for cars (in constrained locations), use a coefficient of deceleration of 0.41. The resultant crest curve size can then be calculated using the relevant equations in AGRD Part 3 (Austroads 2016b).

3 A 1.5 sec reaction time is only to be used in constrained situations where drivers will be alert. Typical situations are given in Table 5.2 of AGRD Part 3. The general minimum reaction time is 2 sec.

4 This check case assumes the same combination of design speed and reaction time as those listed in the table, except that the 120 km/h and 130 km/h speeds are not used.

Notes:

K is the length of vertical curve in metres for a 1% grade change.

Main Roads Western Australia has adopted a desirable minimum reaction time of 2.5 sec and an absolute minimum reaction time of 2.0 sec. A reaction time of 1.5 sec is not to be used in Western Australia.

Combinations of design speed and reaction times not shown in this table are generally not used.

Refer to AGRD Part 3 to determine the ASD for trucks around horizontal curves.

	Based on safe intersection sight distance for cars ⁽¹⁾ $h_1 = 1.1; h_2 = 1.25, d = 0.36^{(2)};$ Observation time = 3 sec					
Design speed (km/h)	$R_T = 1.5 \mathrm{sec}^{(3)}$		<i>R</i> ₇ = 2.0 sec		<i>R</i> ₇ = 2.5 sec	
	SISD (m)	K	SISD (m)	K	SISD (m)	К
40	67	4.9	73	6	-	-
50	90	8.6	97	10	-	-
60	114	14	123	16	-	-
70	141	22	151	25	-	-
80	170	31	181	35	-	-
90	201	43	214	49	226	55
100	234	59	248	66	262	74
110	-	-	285	87	300	97
42uddenha	am Road	-	324	112	341	124
130	-	-	365	143	383	157

 Table 3.2:
 Safe intersection sight distance (SISD) and corresponding minimum crest vertical curve size for sealed roads (S < L)</th>

1 If the average grade over the braking length is not zero, calculate the safe intersection sight distance (SISD) values using the correction factors in Table 3.4 (or use Equation 2) by applying the average grade over the braking length.

2 A coefficient of deceleration of greater than 0.36 is not provided in this table. The provision of SISD requires more conservative values than for other sight distance models (e.g. the stopping sight distance model allows values up to 0.46 in constrained situations). This is because there is a much higher likelihood of colliding with hazards at intersections (that is, other vehicles). Comparatively, there is a relatively low risk of hitting a small object on the road (the stopping sight distance model).

3 A 1.5 sec reaction time is only to be used in constrained situations where drivers will be alert. Typical situations are given in Table 4.2 of AGRD Part 3 (Austroads 2016b). The general minimum reaction time is 2 sec.

Notes:

K is the length of vertical curve for a 1% change in grade.

To determine SISD for trucks around horizontal curves, use Equation 2 with an observation time of 2.5 sec. Main Roads Western Australia have adopted a desirable minimum reaction time of 2.5 sec and an absolute minimum reaction time of 2.0 sec. A reaction time of 1.5 sec is not to be used in Western Australia.

Combinations of design speed and reaction times not shown in this table are generally not used.

Minimum SISD capability provided by the crest vertical curve size ⁽¹⁾	Car at night ⁽²⁾	$d = 0.46$, $h_1 = 0.65$ m, $h_2 = 1.25$ m, observation time = 2.6 sec (car headlight to top of car) $d = 0.46$, $h_1 = 1.1$ m, $h_2 = 0.8$ m, observation time = 2.5 sec (car driver eye height to car taillight)
	Truck	$d = 0.24$, $h_1 = 2.4$ m, $h_2 = 1.25$ m, observation time = 3.0 sec (truck driver height to top of car)
	Truck at night ⁽²⁾	$d = 0.29$, $h_1 = 1.05$ m, $h_2 = 1.25$ m, observation time = 1.8 sec (commercial vehicle headlight to top of car) $d = 0.29$, $h_1 = 2.4$ m, $h_2 = 0.8$ m, observation time = 3.0 sec (truck driver eye height to car taillight)

1 These check cases assume the same combination of design speed and reaction time as those listed in the table, except that the 120 km/h and 130 km/h speeds are not used for the truck cases.

2 Many of the sight distances corresponding to the minimum crest size are greater than the range of most headlights (that is, 120–150 m). In addition, tighter horizontal curvature will cause the light beam to shine off the pavement (assuming 3° lateral spread each way).

Note: Designers should also refer to AGRD Part 3 for further information on the vertical height parameters.

	Design speed (km/h)	Absolute minimum values Only for specific road types and situations ⁽¹⁾ based on <i>d</i> = 0.46 ^{(2),(3)}			Desirable minimum values for all road types based on <i>d</i> = 0.36			Values for major highways and freeways in flat terrain ⁽⁷⁾ based on <i>d</i> = 0.26	
	Road 01	<i>R</i> _T = 1.5 s ⁽⁴⁾	<i>R</i> _T = 2.0 s ⁽⁴⁾	<i>R</i> _T = 2.5 s	<i>R</i> _T = 1.5 s ⁽⁴⁾	<i>R</i> _T = 2.0 s ⁽⁴⁾	<i>R</i> _T = 2.5 s	<i>R</i> _T = 2.0 s	<i>R</i> _T = 2.5 s
	40	30	36	-	34	40	45	-	-
	50	42	49	-	48	55	62	-	-
	6	56	64	-	64	73	81	-	_
	70	71	81	-	83	92	102	113	123
	80	88	99	-	103	114	126	141	152
	90	107	119	132	126	139	151	173	185
	1/1	-	141	155	_	165	179	207	221
	11 <mark>0</mark>	-	165	180	-	193	209	244	260
_	12 <mark>0</mark>		190	207	-	224	241	285	301
ud	denham R	oad -	217	235	-	257	275	328	346
	Corrections due to grade ^{(5) (6)}	-8	-6	-4	-2	2	4	6	8
	40	5	3	2	1	-1	-2	-2	-3
	50	8	5	3	2	-1	-3	-4	-5
	60	11	8	5	2	-2	-4	-6	-7
	70	15	11	7	3	-3	-5	-8	-10
	80	20	14	9	4	-4	-7	-10	-13
	90	25	18	11	5	-5	-9	-13	-16
	100	31	22	14	6	-6	–11	-16	-20
	110	38	26	17	8	-7	-13	-19	-24
	120	45	31	20	9	-8	-16	-22	-29
	130	53	37	23	11	-10	–18	-26	-34

Table 5.5:	Stopping sight distances for cars on sealed roads
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1 These values are only suitable for use in very constrained locations. Examples of this in Australia are: – lower volume roads

mountainous roads

- lower speed urban roads

sighting over or around barriers.

2 On any horizontal curve with a side friction factor greater than the desirable maximum value, reduce the coefficient of deceleration by 0.05 and calculate the stopping distance according to Equation 1.

- 3 Where deceleration values greater than 0.36 are used, minimum seal widths for supplementary manoeuvre capability should be provided. For two-lane, two-way roads, a desirable minimum width of 12 m and a minimum of 9 m is applicable. This is especially important on horizontal curves with a side friction demand greater than the desirable maximum in Table 7.5.
- 4 Reaction times of 1.5 s cannot be used in Western Australia. A 1.5 s reaction time is only to be used in constrained situations where drivers will be alert. Typical situations are given in Table 5.2. The general minimum reaction time is 2.0 s.
- 5 If the roadway is on a grade, designers shall adjust stopping sight distance values by applying these grade corrections derived with d = 0.36. Downhill grades are shown as negative, with uphill listed as positive. The grade adopted is determined using the average grade over the braking length. Grade correction for d = 0.46 should be calculated separately using Equation 1. Generally, grade corrections are not necessary when using d = 0.26 because the deceleration value is conservative and because steep grades are not usually applied to roadways utilising d = 0.26.
- 6 Corrected stopping sight distances should be rounded conservatively to the nearest 5 m.
- 7 Green shaded area of Table 5.5 should only be used with the written approval of the relevant road agency when project objectives are being established.

Note: Combinations of design speed and reaction times not shown in this table are generally not used. Either the resulting stopping distances are similar to other combinations of the parameters for the design speed, or they fall outside the realistic design speed for the road.

7.6 Side Friction and Minimum Curve Size

A vehicle travelling around a circular horizontal curve requires a radial force that tends to effect the change in direction and consequent centripetal acceleration. This force is provided by side friction between the tyres and the road surface. If there is insufficient force provided by side friction, the vehicle will tend to slide tangentially to the road alignment.

The side friction factor (f) is a measure of the frictional force between the pavement and the vehicle tyre. Based on the review of side friction factor values used by Australian and international agencies, the desirable and absolute values of f recommended for design are shown in Table 7.5.

The value of the side friction factor depends on the type and condition of the road surface, driver behaviour and the type and condition of the tyres. Therefore, it is variable.

The desirable maximum values should be used on intermediate and high speed roads with uniform traffic flow, on which drivers are not tolerant of discomfort. Where possible, these values should be adopted to allow vehicles to maintain their lateral position within a traffic lane and to be able to comfortably change lanes if necessary. Figure 7.7 to Figure 7.9 provide the relationship between speed, radius and superelevation using the desirable maximum values of side friction, for both urban and rural roads.

On low speed roads with non-uniform traffic flow, which are typical in urban areas or mountainous terrain, drivers are more tolerant of discomfort. This permits the absolute maximum values of side friction to be safely used in the design of horizontal curves, although the designer should endeavour to adopt desirable maximum values where possible. The minimum radii curves listed in Table 7.6 are suitable in constrained urban areas but their use in rural areas will result in a poor alignment and associated road safety issues.

	f					
Operating speed (km/h)	Ca	ars	Trucks			
(Des max	Abs max	Des max	Abs max		
40	0.30	0.35	0.21	-		
50	0.30	0.35	0.21	0.25		
60	0.24	0.33	0.17	0.24		
70	0.19	0.31	0.14	0.23		
80	0.16	0.26	0.13	0.20		
90	0.13	0.20	0.12	0.15		
100	0.12	0.16	0.12	0.12		
110	0.12	0.12	0.12	0.12		
120	0.11	0.11		umed for 1		
130	0.11	0.11	. Luddenh	am Road_		

Table 7.5: Recommended side friction factors for cars and trucks

Note: ARRB research into the stability of high centre of gravity articulated vehicles indicated that the least stable vehicles may roll over at side friction values as high as 0.35 (Mai & Sweatman 1984).

[see Commentary 16]

7.6.1 Minimum Radius Values

The minimum radius of a horizontal curve for a given operating speed can be determined from Equation 5. Using the values for f_{max} from Table 7.5, the approximate minimum radii for various vehicle speeds for typical values of e_{max} are as shown in Table 7.6.

13.7 Vertical clearance at structures

The minimum vertical clearance to superstructure components of bridges and other structures shall be as given in Table 13.7, unless specified otherwise or agreed by the relevant authority.

TABLE 13.7

MINIMUM VERTICAL CLEARANCE

Location	Clearance m			
Above urban and rural freeways	5.4	Minimum vertical		
Above main and arterial roads	5.4	clearance to viaduct		
Above other roads	4.6 (see Note 3)			
Above high clearance routes	5.9			
Above very high clearance routes (with no alternative)	6.5			
Beneath pedestrian and cyclist path bridges	- At least 0.2 greater than adjac less than 5.4	ent bridges, but not		
	- 5.5 where there are no adjacer	nt bridges		
	- 6.0 on designated high clearar	nce routes		
	 Over navigable waterways at least 0.2 greater than the nearest road or rail bridges upstream or downstream 			
Beneath major overhead sign structures	- 5.4 above any moving traffic bedge of the sign, supporting st mounted below the sign			
	- 5.9 for high clearance routes			
	- 6.0 where future lighting is considered			
Above pedestrian paths	2.4			
Above cyclist paths and shared paths	2.7			
Above rail	See Clause 13.8			
Above light rail	5.3 to be confirmed with the rele	evant authority		
Above or below aerial electricity cables 500 kV 220 kV	17.0 14.5			
Above waterways	See Clause 11.1 and Clause 13.3	3		

NOTES:

- 1 The vertical clearances given in this Table include an allowance of 100 mm for the combined effect of settlement and road resurfacing. Where these effects may be greater than 100 mm, additional vertical clearance shall be provided, as appropriate.
- 2 Vertical bridge clearances shall be designed in accordance with AS 1742.2.
- 3 Provided there is a 5.4 m clearance on an alternative route approved by the relevant authority.

13.8 Bridges over rail

Vertical and horizontal clearances for bridges over rail shall be as required by the rail authority and shall be considered together with the requirements of Clause 15.

13.9 Superelevation and crossfall on road bridges

At each abutment, the geometry of the bridge pavement shall match that of the road approaches. Where stage construction is envisaged, consideration shall be given to the final structural arrangement. The superelevation and widening of the deck surface of a bridge on a horizontal curve shall be as required by the relevant authority.



Appendix D Preliminary Signals Plan

Road Delay Solutions



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	VICUT () () () () () () () () () ()	A channel lets of anno a second equals a second equals
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